GENERAL

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, DESIGN PROFESSIONAL, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE DESIGN PROFESSIONAL OF RECORD OR ANY OF THE DESIGN PROFESSIONAL OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE THE ARCHITECTURAL DRAWINGS.
7. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY.
8. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
9. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.
10. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
11. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
12. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE DESIGN PROFESSIONAL DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE DESIGN PROFESSIONAL. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
13. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE TYPICAL DETAILS UNLESS THOSE LOCATIONS ARE SPECIFICALLY DETAILED OTHERWISE.
14. STRUCTURAL DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR THE DESIGN OF CURTAIN WALL/WINDOW WALL SYSTEMS, COLD-FORMED METAL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.
15. SUBMITTALS

17.1 SUBMITTALS BY THE CONTRACTOR ARE NOT A PART OF THE CONTRACT DOCUMENTS. PRIOR TO THE INITIAL SUBMITTAL, CONTRACTOR SHALL SUBMIT TO THE DESIGN PROFESSIONAL A SCHEDULE OF SUBMITTED INFORMATION.

17.2 SUBMITTALS SHALL BE ACCOMPANIED BY A TRANSMITTAL LETTER WITH THE FOLLOWING INFORMATION:

- PROJECT NAME
- CONTRACTOR'S NAME
- DATE SUBMITTED
- DESCRIPTION OF ITEMS SUBMITTED. IDENTIFY WORK AND PRODUCT BY SPECIFICATION SECTION
- NUMBER OF DRAWINGS AND OTHER PERTINENT DATA.

17.3 CONTRACTOR SHALL DIRECT SPECIFIC ATTENTION ON THE SUBMITTAL TO ANY DEVIATION FROM THE CONTRACT DOCUMENTS. CONTRACTOR SHALL STAMP AND SIGN EACH SHEET OF SHOP DRAWINGS AND PRODUCT DATA, AND SIGN OR INITIAL EACH SAMPLE TO CERTIFY COMPLIANCE WITH REQUIREMENTS OF CONTRACT DOCUMENTS. SUBMITTALS RECEIVED WITHOUT THE CONTRACTOR'S STAMP OF REVIEW WILL BE RETURNED TO THE CONTRACTOR FOR REVIEW AND RESUBMITTAL.

17.4 WORK REQUIRING SHOP DRAWINGS, WHETHER CALLED FOR BY THE CONTRACT DOCUMENTS OR REQUESTED BY THE CONTRACTOR, SHALL NOT COMMENCE UNTIL THE SUBMISSION HAS BEEN REVIEWED BY THE DESIGN PROFESSIONAL. WORK MAY COMMENCE IF THE CONTRACTOR VERIFIES THE ACCURACY OF THE DESIGN PROFESSIONAL'S CORRECTIONS AND NOTATIONS AND COMPLIES WITH THEM WITHOUT EXCEPTION AND WITHOUT REQUESTING CHANGE IN CONTRACT SUM OR CONTRACT TIME AT COPY OF THE MARKED STRUCTURAL SHOP DRAWINGS WITH THE DESIGN PROFESSIONAL'S REVIEW STAMP IS TO BE MAINTAINED AT THE JOB SITE.

CODE/DESIGN CRITERIA

1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FOLLOWING:
   - INTERNATIONAL BUILDING CODE, 2024 EDITION WITH GEORGIA AMENDMENTS.
2. GRAVITY LOADS

2.1 UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

GENERAL AREAS	100 PSF
CLASSROOMS	40 PSF
OFFICES	50 PSF
CORRIDORS - FIRST FLOOR	100 PSF
MECHANICAL ROOMS	150 PSF
STORAGE	125 PSF
STAIRS	100 PSF
WALKWAYS AND ELEVATED PLATFORMS	60 PSF

2.2 UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

- ROOF, L_r 20 PSF
- GROUND SNOW LOAD, P_g 28 PSF
  - SNOW EXPOSURE FACTOR, C_e 0.9
  - WINTER WIND FACTOR, W_2 0.35
- 15-MIN RAINFALL INTENSITY 5.78 in/h

2.4 DEAD LOADS (IN ADDITION TO STRUCTURE SELF-WEIGHT):
ROOF:

- ROOFING 10 PSF
- CEILING/MEP 6 PSF

3. WIND LOADS:
   - BASIC DESIGN WIND SPEED, V = 105 MPH
   - ALLOWABLE DESIGN WIND SPEED, V_{ASD} = 81 MPH
   - RISK CATEGORY: II
   - EXPOSURE C
   - INTERNAL PRESSURE COEFFICIENT = +/-0.18

   SEE COMPONENT AND CLADDING DESIGN WIND PRESSURE DIAGRAM

CODE/DESIGN CRITERIA CONT'D.

4. EARTHQUAKE LOADS:
   - RISK CATEGORY: II
   - SEISMIC IMPORTANCE FACTOR: I = 1.0
   - SHORT PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_S = 0.39
   - 1 SECOND PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_1 = 0.11
   - SITE CLASS D
   - SHORT PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{DS} = 0.29
   - 1 SECOND PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{D1} = 0.15
   - SEISMIC DESIGN CATEGORY: C
   - BASIC SEISMIC-FORCE RESISTING SYSTEM: ORDINARY REINFORCED MASONRY SHEAR WALLS
     - SEISMIC RESPONSE COEFFICIENT, C_S = 0.145
     - RESPONSE MODIFICATION FACTOR, R = 2
     - BASE SHEAR, V = 53 kip (4)
     - SEISMIC ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE
5. UNLESS NOTED OTHERWISE CALCULATED INDIVIDUAL MEMBER DEFLECTIONS (IN INCHES) DO NOT EXCEED THE FOLLOWING:

	DEAD LOAD	LIVE LOAD	DEAD + LIVE LOAD
ROOF MEMBERS:	L/360	L/360	L/240

   - WHERE, L = SPAN LENGTH (IN INCHES) BETWEEN SUPPORTS. (FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER.) NOTE THAT THE TOTAL MAXIMUM CALCULATED FLOOR SYSTEM DEFECTION WILL BE THE SUM OF THE DEFLECTIONS OF THE SUPPORTED ELEMENTS IN A BAY.
   - THE CALCULATED DEFLECTION FOR INDIVIDUAL MEMBERS SUPPORTING MASONRY DO NOT EXCEED L/600 FOR DESIGN LOADS APPLIED AFTER THE INSTALLATION OF THE MASONRY.
6. SPECIAL INSPECTIONS:

6.1 THE STRUCTURAL TESTING/INSPECTION AGENCY, SEE SPECIFICATION SECTION 014525, WILL PERFORM SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE. MATERIALS AND WORK TO BE INSPECTED INCLUDE CONCRETE, MASONRY, STEEL AND SOIL CONSTRUCTION. SEE SCHEDULE FOR A COMPLETE LIST OF WORK REQUIRING SPECIAL INSPECTIONS.

6.2 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE ARE REQUIRED FOR STRUCTURAL COMPONENTS AND ASSEMBLIES WHICH ARE NOT FABRICATED AT THE CONSTRUCTION JOB SITE INCLUDING BUT NOT LIMITED TO ROOF JOISTS OF STEEL MATERIALS, AND STRUCTURAL STEEL FRAMING.

6.3 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE MAY BE WAIVED FOR ITEMS WHICH ARE PRODUCED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND BY PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE WHICH STATES THAT THE FABRICATION WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

6.4 THE DESIGN PROFESSIONAL WILL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PERFORM INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE DURING CONSTRUCTION OF THE PROJECT. DOCUMENTATION THAT SUMMARIZES THE QUALIFICATION AND CREDENTIALS OF EACH SPECIAL INSPECTOR AND DEMONSTRATES COMPETENCE FOR INSPECTION OF EACH PARTICULAR TYPE OF CONSTRUCTION REQUIRING SPECIAL INSPECTION SHALL BE SUBMITTED TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

6.5 APPROVED SPECIAL INSPECTORS SHALL FURNISH INSPECTION REPORTS TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE AND TO THE DESIGN PROFESSIONAL WHICH INDICATE THAT THE WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. A FINAL REPORT WHICH DOCUMENTS THE RESULTS OF THE SPECIAL INSPECTIONS PERFORMED INCLUDING CORRECTION OF ANY DISCREPANCIES IDENTIFIED DURING INSPECTION SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY APPROVED BY THE CHIEF COMMERCIAL BUILDING INSPECTOR PRIOR TO CONSTRUCTION.

6.6 SPECIAL INSPECTION REPORTS AND FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF WORK IS APPROVED FOR OCCUPANCY.

7. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

FOUNDATION

1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY TERRACON, REPORT NUMBER 499245244 DATED DECEMBER 12, 2024. DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR DESIGN.
2. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.
3. INDIVIDUAL SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 3,000 PSF AND 3,000 PSF, RESPECTIVELY.

   3.1 NO FOOTINGS SHALL BEAR ON ROCK. UNDERCUT ROCK A MINIMUM OF 2 FEET BELOW BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL.
4. PROOF ROLL BUILDING AREAS WITH TWO COMPLETE COVERAGES OF A LOADED DUMP-TRUCK OR SCRAPER. REPLACE SOFT AREAS WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.
5. STRUCTURAL FILL SHALL CONTAIN NO ORGANIC MATERIAL AND BE APPROVED BY A GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT. STRUCTURAL FILL UNDER SLABS AND WITHIN 10'-0" OF THE BUILDING FOOTPRINT SHALL BE PLACED IN LIFTS OF THICKNESS DETERMINED BY THE INDEPENDENT TESTING AGENCY AND COMPACTED TO AT LEAST 95% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698. THE TOP 12" SUB-BASE UNDER SLABS ON GRADE SHALL BE COMPACTED TO AT LEAST 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY. ALL BACKFILL, COMPACTION AND PROOF ROLLING OPERATIONS SHALL BE OBSERVED BY AN INDEPENDENT TESTING LABORATORY. STRUCTURAL FILL SOIL DENSITY SHALL BE 120 PCF.
6. SLABS-ON-GRADE SHALL BE PLACED ON A 4" GRANULAR BASE, COMPACTED TO 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698, AND COVERED WITH A CONTINUOUSLY SEALED VAPOR BARRIER. SEE ARCHITECT FOR THICKNESS OF VAPOR BARRIER THE BASE FOR SLABS-ON-GRADE SHALL BE INSPECTED BY A GEOTECHNICAL ENGINEER PRIOR TO EACH PLACEMENT OF CONCRETE.
7. FOOTINGS SHALL BE CENTERED ABOUT COLUMN LINES UNLESS NOTED OTHERWISE.
8. ALL FOOTINGS AND TURN DOWN SLAB EDGES SHALL PENETRATE TO A MINIMUM DEPTH OF 12" BELOW FINISHED GRADE.

REINFORCEMENT

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND HAVE MINIMUM SIDE AND END LAPS OF 8".
3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.
4. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI 318, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE DESIGN PROFESSIONAL.
5. PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING, UNLESS NOTED OTHERWISE.
6. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:

6.1 CONCRETE REINFORCEMENT COVER

EXPOSED TO EARTH OR WEATHER:

- UNFORMED CAST AGAINST EARTH 3" CLEAR
- FORMED #6 AND LARGER 2" CLEAR
- FORMED #5 AND SMALLER 1-1/2" CLEAR

NOT EXPOSED TO EARTH OR WEATHER:

- SLABS 3/4" CLEAR

6.2 MASONRY REINFORCING STEEL SHALL BE PLACED IN THE CENTER OF THE WALL UNLESS NOTED OTHERWISE.

7. REINFORCING STEEL DESIGNATED CONTINUOUS SHALL BE LAPPED AS FOLLOWS:
   - CONCRETE REINFORCEMENT: CLASS B TENSION LAP
   - MASONRY REINFORCEMENT: SEE DETAIL 1/S-401
8. ALL DOWELS AND TERMINATING BARS SHALL HAVE A STANDARD 90 DEGREE HOOK.
9. ALL HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL AND/OR CONSTRUCTION JOINTS AND AROUND CORNERS, UNLESS SHOWN OTHERWISE IN DETAILS.

CAST-IN-PLACE CONCRETE

1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS.
2. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:

2.1 NORMAL WEIGHT STRUCTURAL CONCRETE:

FOOTINGS	3000 PSI	F0
SLABS-ON-GRADE	4000 PSI	F0
EXTERIOR SLABS-ON-GRADE	4500 PSI	F2

3. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE SLAB OR WALL THICKNESS INCLUDING CROSSING UNLESS SPECIFICALLY DETAILED IN THE STRUCTURAL DOCUMENTS. ALL PIPES AND DUCTS SHALL BE PLACED IN THE MIDDLE THIRD OF THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED OTHERWISE IN THE STRUCTURAL DOCUMENTS. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.
4. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.
5. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE DESIGN PROFESSIONAL. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
6. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.016 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE DESIGN PROFESSIONAL.

CONCRETE MASONRY

1. MINIMUM 28-DAY COMPRESSIVE STRENGTH OF ASTM C90 CONCRETE MASONRY UNITS SHALL BE F'M = 2000 PSI.
2. NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY = 2000 PSI.
3. MORTAR SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY AND ASTM C270. MORTAR SHALL BE OF THE FOLLOWING TYPE:
   - WALLS BELOW GRADE TYPE M
   - BEARING WALLS TYPE M OR S
4. CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 2500 PSI COARSE GROUT AS SHOWN IN THE STRUCTURAL DOCUMENTS. GROUT FOR REINFORCED AND NONREINFORCED MASONRY SHALL CONFORM TO ASTM C476.
5. PROVIDE HORIZONTAL LADDER-TYPE JOINT REINFORCEMENT WITH NO. 9 GAGE DEFORMED LONGITUDINAL WIRES AT 16" C/C VERTICALLY AND AT 8" BELOW GRADE, UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC. LONGITUDINAL WIRES SHALL BE PLACED IN THE MORTAR JOINTS.
6. PROVIDE OPEN BOTTOM BEAM BLOCK UNITS WITH 3" DEEP MINIMUM WEB OPENINGS AT HORIZONTAL REINFORCEMENT LOCATIONS. A MINIMUM CLEAR SPACE OF ONE BAR DIAMETER SHALL BE PROVIDED BETWEEN THE REINFORCING BARS AND THE FACE OF MASONRY UNITS.
7. PROVIDE CONTROL JOINTS IN ALL CONCRETE MASONRY WALLS AT LOCATIONS APPROVED BY THE DESIGN PROFESSIONAL AT A MAXIMUM SPACING OF 3 TIMES THE WALL HEIGHT OR 25'-0", WHICHEVER IS LESS.
8. SUBMIT WRITTEN CONSTRUCTION PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.
9. MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE #5@32" CENTERED, UNLESS NOTED OTHERWISE.
10. SUBMIT SHOP DRAWINGS FOR MASONRY REINFORCEMENT IN ACCORDANCE WITH SPECIFICATION SECTION 032000.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, UNLESS NOTED OTHERWISE.
   - STRUCTURAL STEEL HSS SHAPES SHALL CONFORM TO ASTM A500, GRADE C.
   - MISCELLANEOUS PLATES AND CONNECTION MATERIAL SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE.
2. BOLTS AND ANCHORS:

2.1 BOLTED CONNECTIONS SHALL BE TYPE N (BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANE) WITH MINIMUM 3/4" DIAMETER F3125 BOLTS. SUBMIT PROPOSED BOLT TIGHTENING PROCEDURE FOR REVIEW. BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RCSC-2014 (SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS).

2.2 ANCHOR BOLTS SHALL BE HEADED BOLTS CONFORMING TO ASTM F1554 GRADE 36 AND SHALL BE HEADED RODS OR THREADED RODS WITH HEAVY HEXAGONAL NUT WELDED TO THE BOTTOM OF THE THREADED ROD, GRADE A563A, UNLESS NOTED OTHERWISE.

3. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO BOTH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
4. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DESIGNED FOR THE SERVICE LOAD REACTION VALUES SHOWN ON THE STRUCTURAL DRAWINGS. FOR STEEL MEMBERS WHOSE REACTIONS ARE NOT SHOWN, THE DESIGN REACTION SHALL BE DESIGNED FOR THE SERVICE LOAD REACTION OF 10 KIPS UNLESS SHOWN OTHERWISE ON THE STRUCTURAL DRAWINGS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE DESIGN PROFESSIONAL. **DESIGN PROFESSIONAL SHALL BE COMPENSATED BY THE CONTRACTOR FOR THE COST INVOLVED IN THE REDESIGN OF CONNECTIONS FOR THE CONVENIENCE OF THE CONTRACTOR. STEEL CONNECTIONS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. SHOP DRAWINGS AND CALCULATIONS FOR SUCH CONNECTIONS SHALL BE SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.** REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS. FOR CONNECTION DETAILS DEPICTING ARRANGEMENT CONCEPT OF THE CONNECTION WITHOUT COMPLETE DETAILS, THE CONNECTION DESIGN ENGINEER SHALL FOLLOW THAT ARRANGEMENT CONCEPT IN THE DESIGN. SINGLE ANGLE CONNECTIONS ARE NOT ACCEPTABLE.
5. USE PRE-QUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY D1.1/D1.1M-2015. "NON-PRE-QUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.
8. STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED.

STEEL JOISTS

1. STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED, FABRICATED, AND ERECTED ACCORDING TO THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI).
2. STEEL ROOF JOISTS AND BRIDGING SHALL BE DESIGNED FOR A NET UNIFORM UPLIFT LOAD AS INDICATED ON THE COMPONENTS AND CLADDING DIAGRAM MINUS THE SELF WEIGHT OF THE JOIST AND 5 PSF SUPERIMPOSED DEAD LOAD.
3. BRIDGING SHALL BE DESIGNED TO FULLY BRACE TOP CHORD OF JOISTS UNDER SERVICE LOADS FOR ROOF JOISTS NOT BRACED BY STEEL ROOF DECK.
4. DESIGN OF STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR THE DESIGN OF THE STEEL JOISTS, BRIDGING AND THEIR CONNECTIONS. SUBMIT CALCULATIONS SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.
5. CONTRACTOR SHALL COORDINATE THE CONSTRUCTION AND ERECTION OF WALLS, BEAM FRAMING, METAL DECKING, ETC. TO ENSURE COMPATIBILITY OF ROOF AND WALL SYSTEMS CONSIDERING PITCH AND CAMBER OF STEEL JOISTS.
6. ALL JOIST SEATS SHALL BE CAPABLE OF RESISTING A 2,000 LB SERVICE OVERTURNING FORCE.

METAL DECK

1. DECK DESIGN IS BASED ON THE STEEL DECK INSTITUTE DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS.
2. PROVIDE GALVANIZED ROOF DECK WITH THE FOLLOWING MINIMUM PROPERTIES:

1.5 INCH	DEPTH
20 GAGE	THICKNESS
0.229 IN^3/FT	SECTION MODULUS
0.217 IN^4/FT	MOMENT OF INERTIA
50,000 PSI	YIELD STRESS

3. DECK IS SPECIFIED BASED ON A THREE SPAN CONDITION. FURNISH HEAVIER GAGE DECK IF REQUIRED FOR ONE OR TWO SPAN CONDITIONS.

DEFERRED SUBMITTALS

1. DEFERRED SUBMITTALS ARE THOSE PORTIONS OF THE DESIGN WHICH ARE NOT SUBMITTED AT THE TIME OF PERMIT APPLICATION AND WHICH ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD.
2. SUBMITTAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD THROUGH THE ARCHITECT AND GENERAL CONTRACTOR. ONCE THE SUBMITTAL DOCUMENTS HAVE BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS, THE ENGINEER OF RECORD WILL FORWARD THEM TO THE ARCHITECT WITH A NOTATION INDICATING THAT THEY ARE IN GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING. THE ARCHITECT WILL FORWARD THE DEFERRED SUBMITTAL DOCUMENTS TO THE GENERAL CONTRACTOR WHO WILL MAINTAIN ONE SET ON SITE FOR REFERENCE BY THE BUILDING INSPECTOR. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.
3. DEFERRED SUBMITTALS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS, CALCULATIONS, DESIGN LOAD DATA AND SUPPORT REACTIONS OF THE COMPONENTS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.
4. ITEMS THAT ARE SUBMITTED FOR CONSIDERATION AS DEFERRED SUBMITTALS ARE AS FOLLOWS:

   a. PREFABRICATED CANOPIES

CROFT

CROFT & Associates
3380 Blue Springs Road
Kennesaw, Georgia 30144
770.529.7714 (p)
www.croftae.com

SHEAR STRUCTURAL
931 MONROE DRIVE
SUITE A102-491
ATLANTA, GA 30308
(678) 664-8051
SHEARSTRUCTURAL.COM

ISSUED FOR CONSTRUCTION

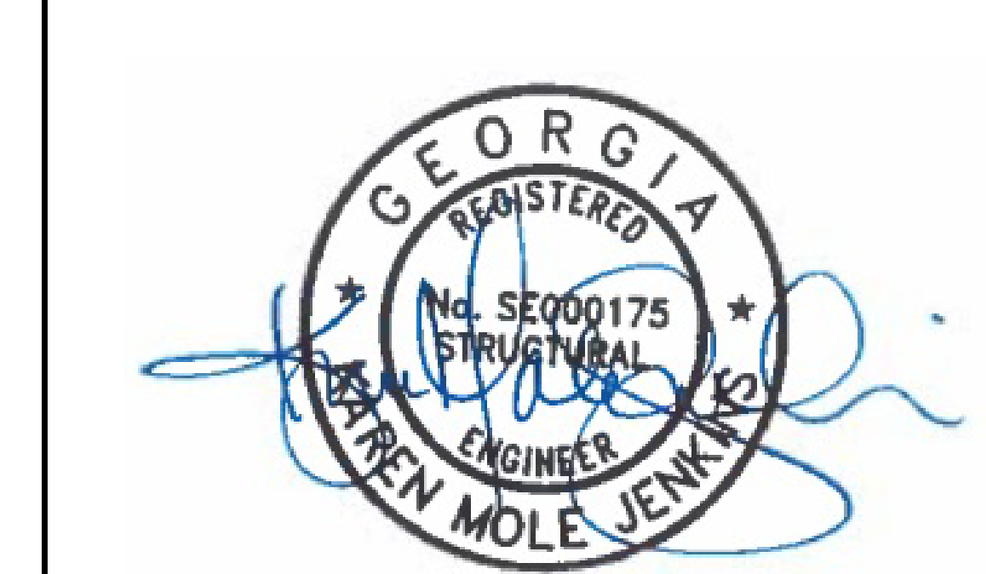
OWNER
GEORGIA STATE FINANCING & INVESTMENT COMMISSION

DANIELA ARSIC
207 WASHINGTON ST SE
ATLANTA, GA 30334

NORTH GEORGIA TECHNICAL COLLEGE

TCSG-410 NORTH GEORGIA TECHNICAL COLLEGE CDL RANGE

121 MEEKS AVE
BLAIRSVILLE, GA 30512



PEF007147
EXP DATE: JUNE 30, 2028

NO.	DATE	DESCRIPTION
5	2026-08-17	FINAL CONSTRUCTION DOCUMENTS
4	2026-06-22	BC #3
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A	2025-02-10	Draft Design Development

ISSUANCE

PROJECT NUMBER
2024-211

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SHEET TITLE
GENERAL NOTES

SHEET NO.
S-001

THIS SHEET PLOTS ON 22" x 34" ANSI D

0 1/2" 1" 2" 3"

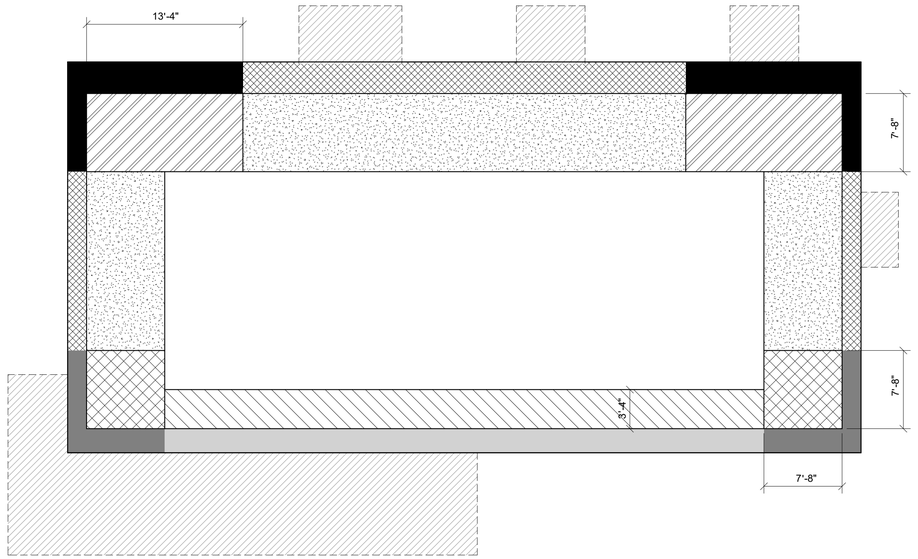
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UPDATED 10/13/23



COMPONENTS AND CLADDING ULTIMATE WIND PRESSURE SCHEDULE (PSF)

EFFECTIVE WIND AREA	ROOF					ROOF OVERHANG				WALL		CANOPY
	ZONE 1	ZONE 2	ZONE 3	ZONE 2'	ZONE 3'	ZONE 2	ZONE 3	ZONE 2'	ZONE 3'	ZONE 4	ZONE 5 (NOTE 2)	
25 SF	-26/+16	-30/+16	-36/+16	-36/+16	-49/+16	-45/+16	-55/+16	-51/+16	-68/+16	-23/+21	-27/+21	-16/+16

SCHEDULE NOTES:
1. (+) AND (-) SIGNS INDICATE PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACE, RESPECTIVELY.
2. PRESSURES APPLY **3'-3"** FROM PROMINENT BUILDING CORNER IN EACH DIRECTION.
3. JOIST MANUFACTURER MAY DEVELOP WIND PRESSURES BASED ON LARGER TRIBUTARY AREAS, DEAD LOAD FOR UPLIFT CALCULATION TO BE 5 PSF.

ABBREVIATIONS

ABT	ABOUT
ACI	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT	ALTERNATE
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL/ARCHITECT
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS
AWS	AMERICAN WELDING SOCIETY
B	BOTTOM OF
BLDG	BUILDING
BM	BEAM
BOTT	BOTTOM
BRG	BEARING
BTWN	BETWEEN
C/C	CENTER TO CENTER
CALC	CALCULATION(S)
CHKD	CHECKED
CIP	CAST-IN-PLACE CONCRETE
CJ	CONSTRUCTION CONTROL JOINT
CJP	COMPLETE JOINT PENETRATION
CL	CENTERLINE
CLR	CLEAR, CLEARANCE
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
COORD	COORDINATE
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
CTR	CENTER
CTRD	CENTERED
DBA	DEFORMED BAR ANCHOR
DBL	DOUBLE
DEG	DEGREES
DET	DETAIL
DIA	DIAMETER
DIAG	DIAGONAL
DIR	DIRECTION
DL	DEAD LOAD
DWG	DRAWING
(E)	EXISTING
EA	EACH
EF	EACH FACE
EL	ELEVATION
ELEC	ELECTRICAL
ENGR	ENGINEER
EOD	EDGE OF DECK
EOR	ENGINEER OF RECORD
EOS	EDGE OF SLAB
EQ	EQUAL
EQUIP	EQUIPMENT
EW	EACH WAY
EXP	EXPANSION
EXT	EXTERIOR
FD	FLOOR DRAIN
FDN	FOUNDATION
FF	FINISHED FLOOR
FG	FINISHED GRADE
FIN	FINISH
FLG	FLANGE
FLR	FLOOR
FRMG	FRAMING
FRP	FIBER REINFORCED PLASTIC
FS	FAR SIDE
FT	FOOT
FTG	FOOTING
FV	FIELD VERIFY
GA	GAGE, GAUGE
GALV	GALVANIZED (HOT DIP)
GRTG	GRATING
(H)	HORIZONTAL BEAM ORIENTATION
HCA	HEADED CONCRETE ANCHOR
HDR	HEADER
HGR	HANGER
HORIZ	HORIZONTAL
HP	HIGH POINT
HR	HANDRAIL
HSB	HIGH STRENGTH BOLT
IF	INSIDE FACE
IBC	INTERNATIONAL BUILDING CODE
ICC	INTERNATIONAL CODE COUNCIL
ID	INSIDE DIAMETER
IE	INVERT ELEVATION
IN	INCH
INT	INTERIOR
JT	JOINT
K	KIP(S)
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
(LLH)	LONG LEG HORIZONTAL (ANGLE)
(LLV)	LONG LEG VERTICAL (ANGLE)
(LSH)	LONG SIDE HORIZONTAL (HSS)
(LSV)	LONG SIDE VERTICAL (HSS)
LB	POUND
LF	LINEAR FEET
LL	LIVE LOAD
LOC	LOCATION
LONG	LONGITUDINAL
LP	LOW POINT
LSH	LONG SLOTTED HOLE
LWC	LIGHT WEIGHT CONCRETE
M	MOMENT
MAX	MAXIMUM
MC	MOMENT CONNECTION
MECH	MECHANICAL
MFC	MANUFACTURED
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
MTD	MOUNTED
MTL	METAL
N&F	NEAR AND FAR
N/A	NOT APPLICABLE
NIC	NOT IN CONTRACT
NO/NO.	NUMBER
NOM	NOMINAL
NS	NEAR SIDE
NTS	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OPNG	OPENING
OPP	OPPOSITE
OSH	OVERSIZED HOLE
PCF	POUNDS PER CUBIC FOOT
PE	PROFESSIONAL ENGINEER
PERIM	PERIMETER
PJF	PREMOLDED JOINT FILLER
PL	PLATE
PLCS	PLACES
PLF	POUNDS PER LINEAR FOOT
PREFAB	PREFABRICATED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POINT
R	RADIUS
RD	ROOF DRAIN
REF	REFERENCE
REINF	REINFORCING
REQD	REQUIRED
RET	RETURN
REV	REVISION
RO	ROUGH OPENING
RTU	ROOFTOP UNIT
SC	SLIP CRITICAL
SCHED	SCHEDULE
SECT	SECTION
SHT	SHEET
SIM	SIMILAR
SL	SLOPE
SPCS	SPACES
SPEC(S)	SPECIFICATION(S)
SQ	SQUARE
SS	STAINLESS STEEL
SSH	SHORT SLOTTED HOLE
STD	STANDARD
STIF	STIFFENER
STL	STEEL
STRUCT	STRUCTURAL
SYM	SYMMETRICAL
T&B	TOP AND BOTTOM
T/O	TOP OF
THK	THICK
THRU	THROUGH
TYP	TYPICAL
UL	UNDERWRITER'S LABORATORIES
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W	WITH
W/O	WITHOUT
WP	WORKING POINT
WS	WATERSTOP
WWF	WELDED WIRE FABRIC

2 S-002 ABBREVIATIONS
3/4" = 1'-0"

STEEL COLUMN/FOOTING TYPE INDICATOR

COL	SIZE STEEL COLUMN SIZE
BP-X	BASE PLATE MARK
FX.X (-0'-0")	FOOTING MARK (T/FTG ELEVATION)
P-X (-0'-0")	PEDESTAL MARK (T/PEDESTAL ELEVATION)

FOUNDATION STEP INDICATOR

(-x'-x") T/FOOTING ELEVATION
(-x'-x") T/FOOTING ELEVATION

OPENING IN FLOOR OR ROOF

RECESS/DEPRESSION INDICATOR

STRUCTURAL STEEL CONNECTION AXIAL FORCE

(xxk)

SLOPE INDICATOR

Y
X

CONCRETE SLAB/METAL DECK SPAN INDICATOR

STRUCTURAL STEEL MOMENT CONNECTION

STRUCTURAL STEEL BEAM DESIGNATION

W8x10 (XX) c=x"
(XXk) (XXk)

BEAM SIZE
NUMBER OF STUDS UNIFORMLY SPACED ALONG BEAM
BEAM CAMBER
BEAM SERVICE REACTION

STRUCTURAL STEEL BEAM SPLICE DESIGNATION

CL
SPLICE

STRUCTURAL CONCRETE BEAM DESIGNATION

CB XXX PB XXX

CBX INDICATES CONVENTIONALLY REINFORCED CONCRETE BEAM MARK BEAM WIDTH MAY VARY BASED UPON EDGE OF SLAB DIMENSION

PBX INDICATES POST-TENSIONED CONCRETE BEAM MARK, BEAM WIDTH MAY VERY BASED UPON EDGE OF SLAB DIMENSION

1 S-002 STRUCTURAL LEGEND
3/4" = 1'-0"

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GEORGIA REGISTERED No. SE000175 STRUCTURAL ENGINEER KAREN MOLE JENKINS
8.17.2026
PEF007147
EXP DATE: JUNE 30, 2028

NO.	DATE	DESCRIPTION
5	2026-08-17	FINAL CONSTRUCTION DOCUMENTS
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ISSUANCE		

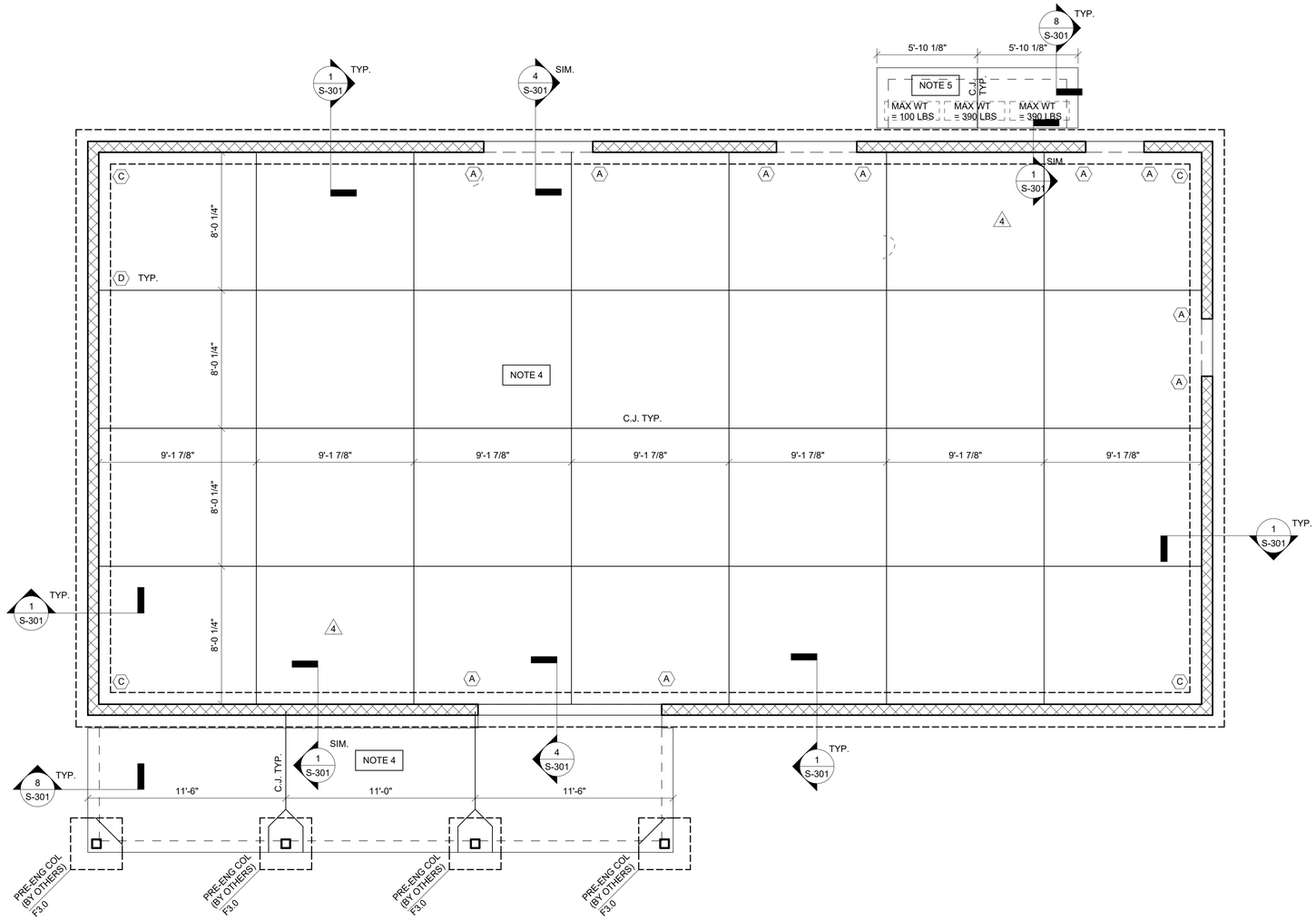
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SHEET TITLE
ABBREVIATIONS, WIND DIAGRAM & LEGEND

SHEET NO.
S-002

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1 S-101 Classroom Foundation
1/4" = 1'-0"

PLAN NORTH

NOTES:
1. SEE S-001 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. Fx INDICATES COLUMN FOOTING. SEE 5/S-301. T/FTG = -1'-0" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0".
4. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 4" GRANULAR BASE.
5. PROVIDE 6" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.9xW2.9 ON VAPOR BARRIER AND 4" GRANULAR BASE AT MECHANICAL PAD. COORDINATE LOCATION WITH ARCH.
6. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S-301 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
7. PROVIDE ISOLATION JOINT AT COLUMN. SEE 6/S-301.
8. ▨ INDICATES MASONRY WALL REINFORCED W/ #5@32" O.C. T/FTG = -1'-4". SEE DETAILS 1,2,3,5 AND 10/S-401.
9. Ⓢ INDICATES MASONRY WALL REINFORCEMENT. SEE DETAIL 5/S-401.
10. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 10/S-301.

PRELIMINARY ALLOWABLE DESIGN COLUMN LOADS			
LOCATION	GRAVITY (KIPS)	UPLIFT (KIPS)	THRUST (KIPS)
INTERIOR	2 (KIPS)	2 (KIPS)	.2 (KIPS)
END	1 (KIPS)	1 (KIPS)	.2 (KIPS)

NOTE: LOADS TO BE VERIFIED W/PRE-ENGINEERED CANOPY MANUFACTURER PRIOR TO CONSTRUCTION.

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JOSEPH MOLE JENKINS
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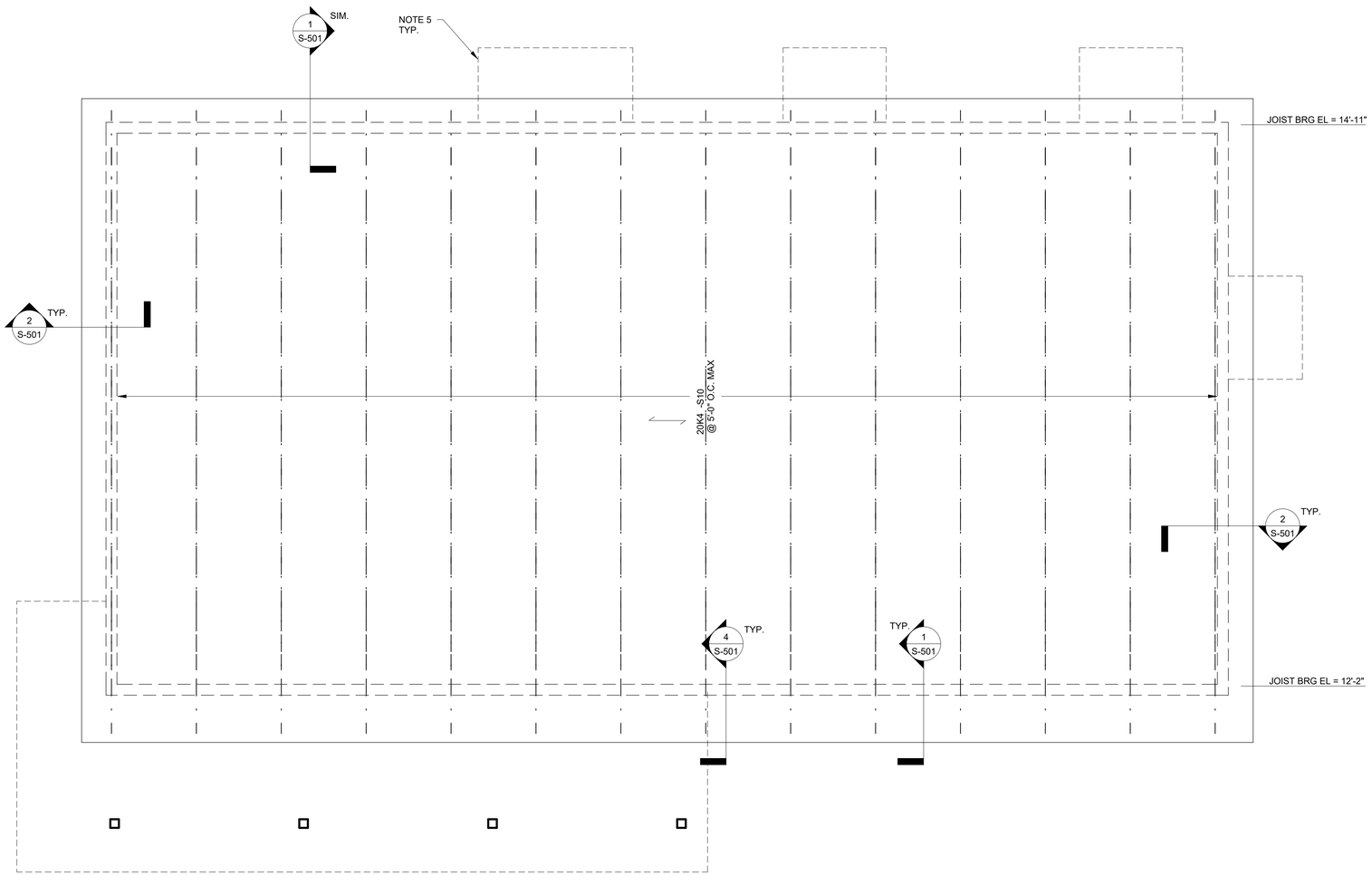
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SHEET TITLE
CLASSROOM FOUNDATION PLAN

SHEET NO.
S-101

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1 S-201 Classroom Roof Framing
1/4" = 1'-0"

PLAN NORTH

NOTES:
1. SEE S-001 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. JOIST BEARING ELEVATION AS INDICATED ON PLAN IS RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" UNO.
4. INDICATES DIRECTIONAL SPAN OF 1 1/2" 20 GA ROOF DECK, SEE 3/S-501
5. INDICATES APPROXIMATE LOCATION OF PREFABRICATED CANOPY. SEE ARCH FOR ELEVATION AND SUPPORT LOCATIONS. CANOPY ARM CONNECTION TO MASONRY BY OTHERS. SEE CONDITION A2 DETAIL 4/S-401 FOR MASONRY REINFORCEMENT AT SUPPORT ARM LOCATIONS.
6. ALL EXPOSED STEEL TO BE GALVANIZED.
7. SEE DETAILS 1, 2, AND 10/S-401 FOR CMU LINTEL SCHEDULE AND DETAILS. COORDINATE ALL OPENING LOCATIONS WITH ARCH.

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EXP DATE: JUNE 30, 2028

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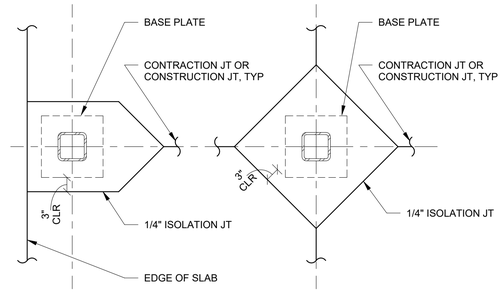
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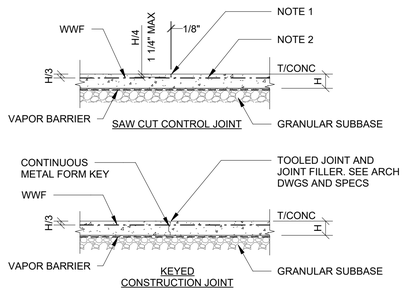
SHEET TITLE
CLASSROOM ROOF FRAMING PLAN

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S-201

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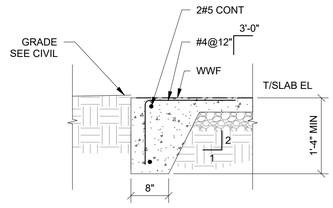
6 S-301 TYPICAL ISOLATION JOINT AT STEEL COLUMN
3/4" = 1'-0"



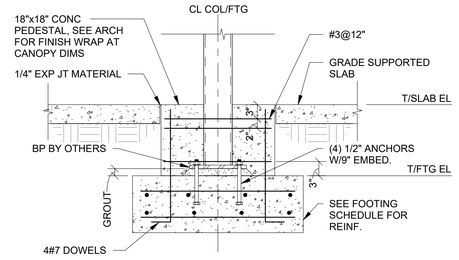
NOTE:
1. SAW CUT SHALL BE MADE SOON ENOUGH TO PREVENT CRACKING, BUT NOT SO SOON AS TO CAUSE SPALLING OF THE CONCRETE WHILE SAWING. FOR JOINT FILLER, SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. FOR JOINT SPACING, SEE STRUCTURAL SPECIFICATIONS.
2. SAW CUT OR DISCONTINUE REINF AT CONSTRUCTION JT.
3. KEYED CONSTRUCTION JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING POURING.
4. WHERE REQUIRED FOR SUCCESSFUL INSTALLATION OF FLOOR FINISH SLAB SHALL BE WET CURED AND BE PLACED A MINIMUM OF 90 DAYS PRIOR TO INSTALLATION OF FLOOR FINISHES. AT CONTRACTOR'S OPTION DIAMOND DOWELS MAY BE USED IN LIEU OF CONTINUOUS CONCRETE KEY AT KEYED JOINT.
5. PROVIDE SLAB BOLSTERS A MAX 2'-0" APART FOR WWF SUPPORT

3 S-301 TYPICAL GRADE SUPPORTED SLAB AT JOINTS
3/4" = 1'-0"

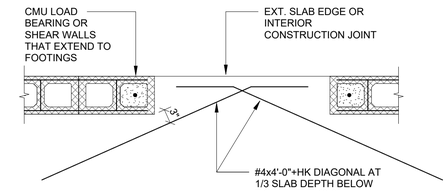
FOOTING SCHEDULE		
MARK	SIZE (WxLxT)	REINF
F3.0	3'-0"x3'-0"x1'-0"	4#4 EW TOP & BOTTOM
F4.0	4'-0"x4'-0"x1'-0"	6#4 EW TOP & BOTTOM



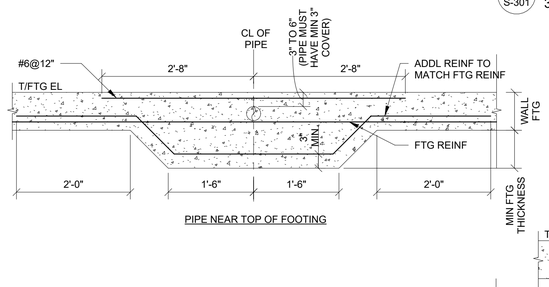
8 S-301 TYPICAL EXTERIOR SLAB TURNDOWN
3/4" = 1'-0"

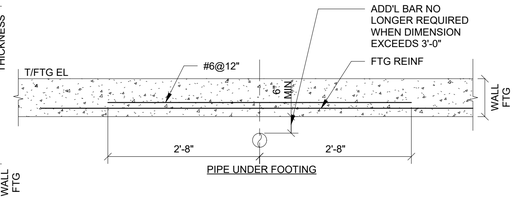


5 S-301 TYPICAL STEEL CANOPY COLUMN FOOTING
3/4" = 1'-0"



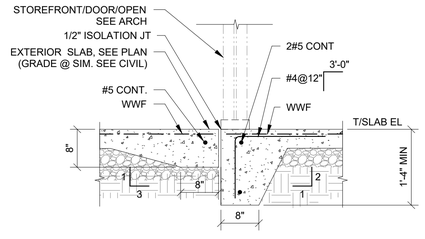
2 S-301 TYPICAL RE-ENTRANT BARS AT CMU WALLS
3/4" = 1'-0"



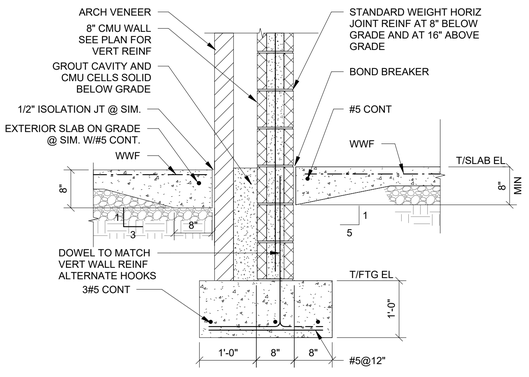


NOTES:
1. NO PIPES TO PASS IN OR BENEATH COLUMN FOOTINGS.
2. PIPES SHALL NOT EXCEED 1/3 THE FOOTING THICKNESS AFTER IT HAS BEEN INCREASED.
3. VERIFY PIPE PENETRATION LOCATIONS W/ PLUMBING CONTRACTOR.

10 S-301 TYPICAL THICKENED FOOTING AT UNDERGROUND PIPING
3/4" = 1'-0"



4 S-301 TYPICAL INTERIOR SLAB TURNDOWN
3/4" = 1'-0"



1 S-301 TYPICAL EXTERIOR 8" CMU WALL FOOTING
3/4" = 1'-0"

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STEPHEN MOLE JENKINS
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S-301

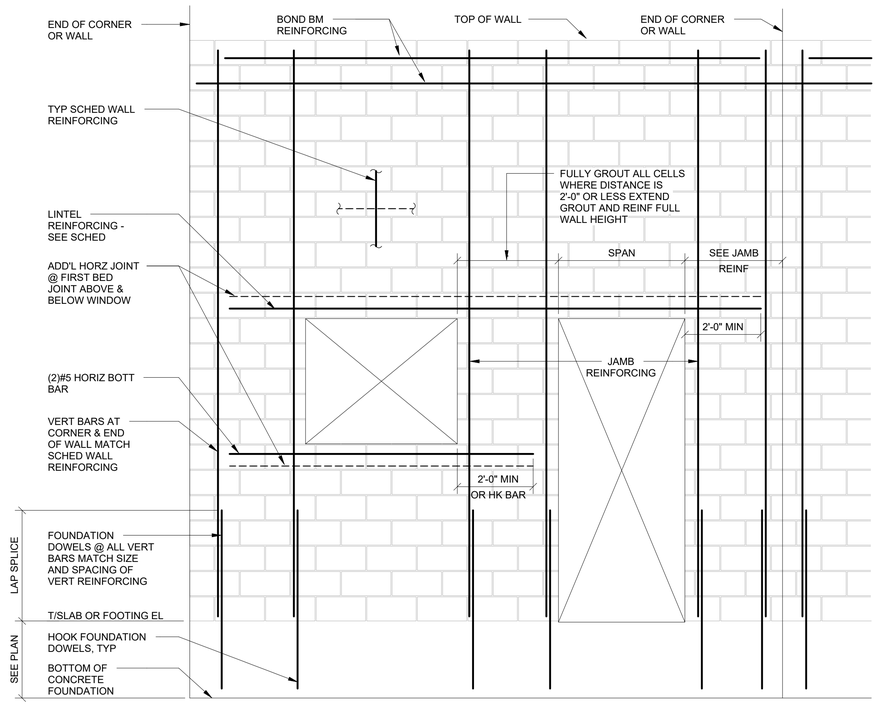
THIS SHEET PLOTS ON 22" x 34" ANSI D

0 1/2" 1" 2" 3"

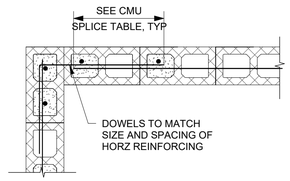
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UPDATED 5/1/023

NOTES

1. ALL REINFORCING SHOWN IN ADDITION TO SCHEDULED WALL REINFORCEMENT.
2. ALL REINFORCING SHALL BE SOLID GROUTED USING 2500 PSI (SPEC COURSE GROUT).
3. REFER TO PLANS, GENERAL NOTES, AND WALL ELEVATIONS FOR TYPICAL VERTICAL AND HORIZONTAL WALL REINFORCEMENT.
4. SEE PLAN FOR LOCATION OF CONTROL JOINTS.
5. USE VERT BAR POSITIONERS AT 4 FT.
6. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAM BLOCKS EXCEPT JOINT REINFORCING.
7. HORIZONTAL REINFORCEMENT SHALL BE DISCONTINUOUS THROUGH CONTROL JOINTS EXCEPT BOND BEAMS AT ROOF, FLOORS AND AT MASONRY LINTELS.
8. PROVIDE HORIZONTAL JOINT REINFORCEMENT AT FIRST BED JOINT ABOVE AND BELOW OPENINGS AND EXTEND 1'-0" BEYOND THE OPENING.
9. COORDINATE LOCATION OF SOUND BLOCK IF SPECIFIED WITHIN REINFORCED CMU WALLS WITH ARCHITECTURAL INTERIOR ELEVATION DRAWINGS.

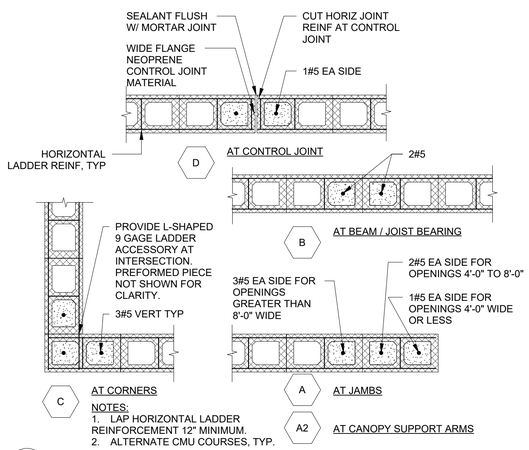


10 S-401 REINFORCEMENT MASONRY WALL ELEVATION
1/2" = 1'-0"



SINGLE LAYER REINFORCING

5 S-401 TYPICAL MASONRY BOND BEAM REINFORCING DETAIL
3/4" = 1'-0"



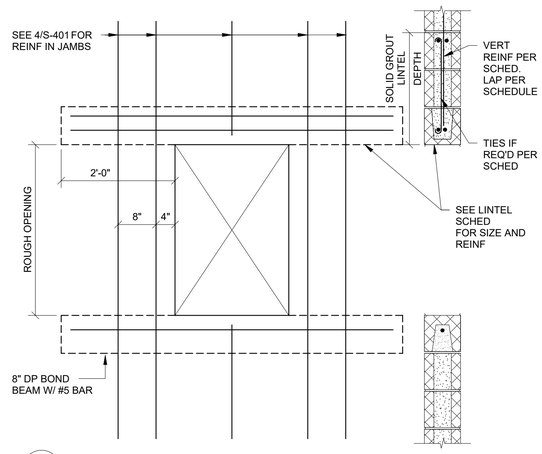
NOTES:
1. LAP HORIZONTAL LADDER REINFORCEMENT 12" MINIMUM.
2. ALTERNATE CMU COURSES, TYP.

4 S-401 TYPICAL REINFORCING AT 8" LOAD BEARING CMU WALL
3/4" = 1'-0"

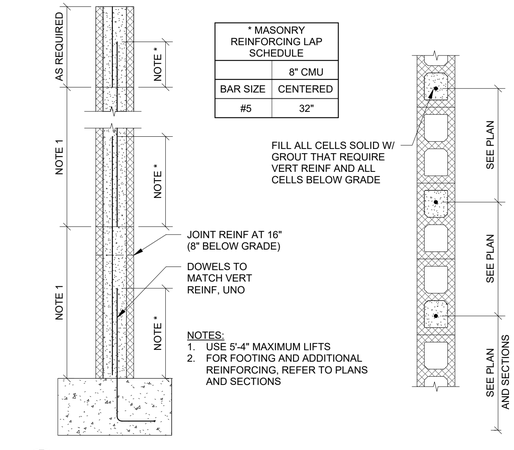
STANDARD LINTEL SCHEDULE		
WALL SIZE	LINTEL TYPE	REMARKS
OPENINGS UP TO 6'-0"		
4" BRICK	5/16"x5"xHORZ LEG BENT PL	ARCH TO PROVIDE HORZ DIM.
4" BLOCK	4"x8" CONC W/ 1#4 T&B	
8" BLOCK	8"x8" CONC W/ 2#4 T&B 8"x8" U-BLOCK W/ 1#5 T&B	
OPENINGS 6'-0" TO 8'-0"		
4" BRICK	3/8"x6"xHORZ LEG BENT PL	ARCH TO PROVIDE HORZ DIM.
8" BLOCK	8"x8" CONC W/ 2#4 T&B 8"x16" U-BLOCK W/ 1#6 T&B	
OPENINGS 8'-0" TO 10'-0"		
6" BLOCK	6"x16" CONC W/ 2#5 T&B	#3 STIR @ 7" CONT
8" BLOCK	8"x8" CONC W/ 2#4 T&B	#3 STIR @ 7" CONT

NOTES:
1. ALL STEEL LINTELS TO BE GALVANIZED.
2. PROVIDE MINIMUM OF 6" BEARING FOR BRICK LINTELS.
3. SEE DETAIL 10/S-401 FOR MINIMUM BEARING OF BLOCK LINTELS.
4. ALL OPENINGS ARE TO BE SHORED UNTIL THE MASONRY HAS CURED FOR A MINIMUM OF 72 HOURS.

3 S-401 STANDARD LINTEL SCHEDULE
12" = 1'-0"



2 S-401 TYPICAL REINFORCING AT CMU WALL OPENING
3/4" = 1'-0"



* MASONRY REINFORCING LAP SCHEDULE	
	8" CMU
BAR SIZE	CENTERED
#5	32"

FILL ALL CELLS SOLID W/ GROUT THAT REQUIRE VERT REINF AND ALL CELLS BELOW GRADE

NOTES:
1. USE 5'-4" MAXIMUM LIFTS
2. FOR FOOTING AND ADDITIONAL REINFORCING, REFER TO PLANS AND SECTIONS

1 S-401 TYPICAL REINFORCED CMU WALL
3/4" = 1'-0"

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121 MEEKS AVE
BLAIRSVILLE, GA 30512

GEORGIA REGISTERED No. SE000175 PRINCIPAL ENGINEER LAUREN MOLE JENKINS
8.17.2026
PEF007147
EXP DATE: JUNE 30, 2028

NO.	DATE	DESCRIPTION
5	2026-08-17	FINAL CONSTRUCTION DOCUMENTS
4	2026-06-22	BC #3
3	2026-03-10	Revised Construction Documents
A	2025-02-10	Draft Design Development

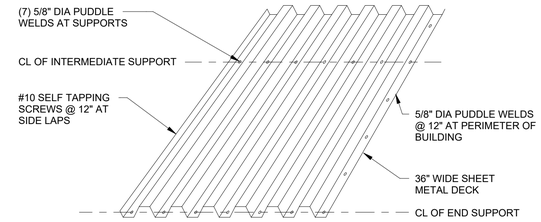
ISSUANCE

PROJECT NUMBER
2024-211

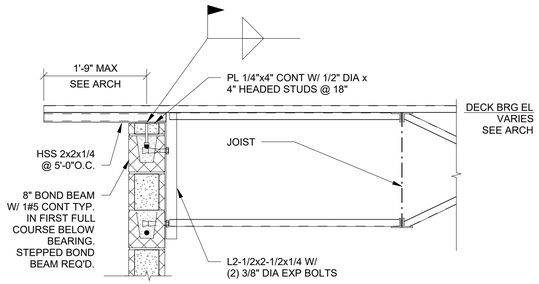
DRAWN: Author
CHECKED: Checker

SHEET TITLE
MASONRY DETAILS

SHEET NO.
S-401

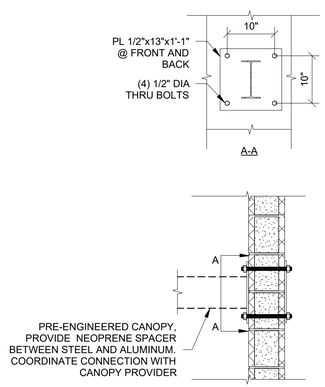


3 S-501 TYPICAL ROOF DECK ATTACHMENT (36" WIDE SHEET)
3/4" = 1'-0"

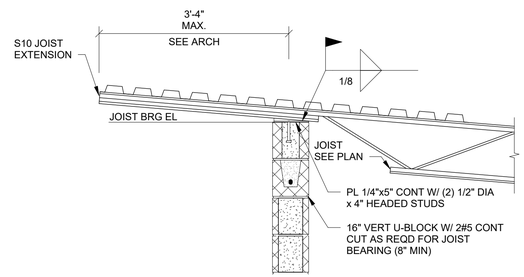


NOTES:
1. VERTICAL REINFORCEMENT NOT SHOWN FOR CLARITY.

2 S-501 TYPICAL 8" CMU WALL AT JOIST
3/4" = 1'-0"



4 S-501 PRE-ENGINEERED CANOPY SECTION AT MASONRY WALL
3/4" = 1'-0"



NOTES:
1. VERTICAL REINFORCEMENT NOT SHOWN FOR CLARITY.

1 S-501 TYPICAL 8" CMU WALL AT JOIST
3/4" = 1'-0"

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3	2026-03-10	Revised Construction Documents
A	2025-02-10	Draft Design Development
ISSUANCE		

PROJECT NUMBER
2024-211

DRAWN: Author
CHECKED: Checker

SHEET TITLE
STEEL FRAMING DETAILS

SHEET NO.
S-501

THIS SHEET PLOTS ON 22" x 34" ANSI D
3" 2" 1" 1/2" 0
8/17/2026 11:34:36 AM Autodesk Docs://2024-211 TCSG-410 NGTC CDL Range/TCSG-410 NGTC CDL Range Struct.rvt
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UPDATED 10/17/23