GENERAL

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, DESIGN PROFESSIONAL, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE DESIGN PROFESSIONAL OF RECORD OR ANY OF THE DESIGN PROFESSIONAL OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE THE ARCHITECTURAL DRAWINGS.
7. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY.
8. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
9. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.
10. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
11. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
12. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE DESIGN PROFESSIONAL DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE DESIGN PROFESSIONAL. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
13. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE TYPICAL DETAILS UNLESS THOSE LOCATIONS ARE SPECIFICALLY DETAILED OTHERWISE.
14. STRUCTURAL DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR THE DESIGN OF CURTAIN WALL/WINDOW WALL SYSTEMS, COLD-FORMED METAL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.
15. SUBMITTALS

    15.1 SUBMITTALS BY THE CONTRACTOR ARE NOT A PART OF THE CONTRACT DOCUMENTS. PRIOR TO THE INITIAL SUBMITTAL, CONTRACTOR SHALL SUBMIT TO THE DESIGN PROFESSIONAL A SCHEDULE OF SUBMITTED INFORMATION.

    15.2 SUBMITTALS SHALL BE ACCOMPANIED BY A TRANSMITTAL LETTER WITH THE FOLLOWING INFORMATION:
    - PROJECT NAME
    - CONTRACTOR'S NAME
    - DATE SUBMITTED
    - DESCRIPTION OF ITEMS SUBMITTED. IDENTIFY WORK AND PRODUCT BY SPECIFICATION SECTION
    - NUMBER OF DRAWINGS AND OTHER PERTINENT DATA.

    15.3 CONTRACTOR SHALL DIRECT SPECIFIC ATTENTION ON THE SUBMITTAL TO ANY DEVIATION FROM THE CONTRACT DOCUMENTS. CONTRACTOR SHALL STAMP AND SIGN EACH SHEET OF SHOP DRAWINGS AND PRODUCT DATA, AND SIGN OR INITIAL EACH SAMPLE TO CERTIFY COMPLIANCE WITH REQUIREMENTS OF CONTRACT DOCUMENTS. SUBMITTALS RECEIVED WITHOUT THE CONTRACTOR'S STAMP OF REVIEW WILL BE RETURNED TO THE CONTRACTOR FOR REVIEW AND RESUBMITTAL.

    15.4 WORK REQUIRING SHOP DRAWINGS, WHETHER CALLED FOR BY THE CONTRACT DOCUMENTS OR REQUESTED BY THE CONTRACTOR, SHALL NOT COMMENCE UNTIL THE SUBMISSION HAS BEEN REVIEWED BY THE DESIGN PROFESSIONAL. WORK MAY COMMENCE IF THE CONTRACTOR VERIFIES THE ACCURACY OF THE DESIGN PROFESSIONAL'S CORRECTIONS AND NOTATIONS AND COMPLIES WITH THEM WITHOUT EXCEPTION AND WITHOUT REQUESTING CHANGE IN CONTRACT SUM OR CONTRACT TIME AT COPY OF THE MARKED STRUCTURAL SHOP DRAWINGS WITH THE DESIGN PROFESSIONAL'S REVIEW STAMP IS TO BE MAINTAINED AT THE JOB SITE.

CODE/DESIGN CRITERIA

1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FOLLOWING:
    - INTERNATIONAL BUILDING CODE, 2024 EDITION WITH 2026 GEORGIA AMENDMENTS.
2. GRAVITY LOADS

    2.1 UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

• GENERAL AREAS	100 PSF
• CLASSROOMS	40 PSF
• OFFICES	50 PSF
• CORRIDORS - FIRST FLOOR	100 PSF
• MECHANICAL ROOMS	150 PSF
• STORAGE	125 PSF

    2.2 UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

• ROOF, L_r	20 PSF
• GROUND SNOW LOAD, P_g	20 PSF
• SNOW EXPOSURE FACTOR, C_e	0.9
• WINTER WIND FACTOR, W_2	0.35
• 15-MIN RAINFALL INTENSITY	6.87 IN/HR

    2.3 DEAD LOADS (IN ADDITION TO STRUCTURE SELF-WEIGHT):

    ROOF:

• ROOFING	10 PSF
• CEILING/MEP	6 PSF

3. WIND LOADS:

• BASIC DESIGN WIND SPEED, V	105 MPH
• ALLOWABLE DESIGN WIND SPEED, V_{ASD}	81 MPH
• RISK CATEGORY	II
• EXPOSURE	C
• INTERNAL PRESSURE COEFFICIENT	+/- 0.18

    SEE COMPONENT AND CLADDING DESIGN WIND PRESSURE DIAGRAM ON SHEET S-002

CODE/DESIGN CRITERIA CONT'D.

4. EARTHQUAKE LOADS:
    - RISK CATEGORY: II
    - SEISMIC IMPORTANCE FACTOR: I = 1.0
    - SHORT PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_S = 0.28
    - 1 SECOND PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_1 = 0.097
    - SITE CLASS D
    - SHORT PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{DS} = 0.23
    - 1 SECOND PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{D1} = 0.14
    - SEISMIC DESIGN CATEGORY: C

    - CLASSROOM BASIC SEISMIC-FORCE RESISTING SYSTEM: ORDINARY REINFORCED MASONRY SHEAR WALLS
        - SEISMIC RESPONSE COEFFICIENT, C_S = 0.115
        - RESPONSE MODIFICATION FACTOR, R = 2
        - BASE SHEAR, V = 42 kips
        - SEISMIC ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE

    5 (revision)
    - PRE-ENGINEERED CANOPY BASIC SEISMIC-FORCE RESISTING SYSTEM: STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC AS PROVIDED BY THE PRE-ENGINEERED CANOPY SUPPLIER
        - SEISMIC RESPONSE COEFFICIENT, C_S = 0.077
        - RESPONSE MODIFICATION FACTOR, R = 3
        - BASE SHEAR, V = 0.7 kips
        - SEISMIC ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE
5. UNLESS NOTED OTHERWISE CALCULATED INDIVIDUAL MEMBER DEFLECTIONS (IN INCHES) DO NOT EXCEED THE FOLLOWING:

	DEAD LOAD	LIVE LOAD	DEAD + LIVE LOAD
ROOF MEMBERS:	L/360	L/360	L/240

    - WHERE, L = SPAN LENGTH (IN INCHES) BETWEEN SUPPORTS. (FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER.) NOTE THAT THE TOTAL MAXIMUM CALCULATED FLOOR SYSTEM DEFECTION WILL BE THE SUM OF THE DEFLECTIONS OF THE SUPPORTED ELEMENTS IN A BAY.
    - THE CALCULATED DEFLECTION FOR INDIVIDUAL MEMBERS SUPPORTING MASONRY DO NOT EXCEED L/600 FOR DESIGN LOADS APPLIED AFTER THE INSTALLATION OF THE MASONRY.
6. SPECIAL INSPECTIONS:

    6.1 THE STRUCTURAL TESTING/INSPECTION AGENCY, SEE SPECIFICATION SECTION 014525, WILL PERFORM SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE. MATERIALS AND WORK TO BE INSPECTED INCLUDE CONCRETE, MASONRY, STEEL AND SOIL CONSTRUCTION. SEE SCHEDULE FOR A COMPLETE LIST OF WORK REQUIRING SPECIAL INSPECTIONS.

    6.2 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE ARE REQUIRED FOR STRUCTURAL COMPONENTS AND ASSEMBLIES WHICH ARE NOT FABRICATED AT THE CONSTRUCTION JOB SITE INCLUDING BUT NOT LIMITED TO ROOF JOISTS OF STEEL MATERIALS, AND STRUCTURAL STEEL FRAMING.

    6.3 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE MAY BE WAIVED FOR ITEMS WHICH ARE PRODUCED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND BY PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE WHICH STATES THAT THE FABRICATION WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

    6.4 THE DESIGN PROFESSIONAL WILL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PERFORM INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE DURING CONSTRUCTION OF THE PROJECT. DOCUMENTATION THAT SUMMARIZES THE QUALIFICATION AND CREDENTIALS OF EACH SPECIAL INSPECTOR AND DEMONSTRATES COMPETENCE FOR INSPECTION OF EACH PARTICULAR TYPE OF CONSTRUCTION REQUIRING SPECIAL INSPECTION SHALL BE SUBMITTED TO THE DESIGN PROFESSIONAL OR HIS DESIGNEE FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

    6.5 APPROVED SPECIAL INSPECTORS SHALL FURNISH INSPECTION REPORTS TO THE DESIGN PROFESSIONAL OR HIS DESIGNEE WHICH INDICATE THAT THE WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. A FINAL REPORT WHICH DOCUMENTS THE RESULTS OF THE SPECIAL INSPECTIONS PERFORMED INCLUDING CORRECTION OF ANY DISCREPANCIES IDENTIFIED DURING INSPECTION SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY APPROVED BY THE DESIGN PROFESSIONAL PRIOR TO CONSTRUCTION.

    6.6 SPECIAL INSPECTION REPORTS AND FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF WORK IS APPROVED FOR OCCUPANCY.
7. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

FOUNDATION

1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY TERRACON, REPORT NUMBER 49255272 DATED OCTOBER 31,2025. DATA CONCERNING SUBSURFACE MATERIALS OR CONDITIONS WHICH ARE BASED UPON SOUNDINGS, TEST PITS, TEST BORINGS, OR OTHER MEANS HAVE BEEN OBTAINED BY THE DESIGN PROFESSIONAL FOR ITS USE IN DESIGNING THE PROJECT. THE ACCURACY OR COMPLETENESS OF THE DATA IS NOT GUARANTEED; AND THEREFORE, THE CONSTRUCTION PROFESSIONAL SHALL NOT RELY ON THIS INFORMATION WITHOUT INDEPENDENT VERIFICATION. NEITHER (LIST INSTITUTION NAME), THE GEORGIA STATE FINANCING AND INVESTMENT COMMISSION, NOR DESIGN PROFESSIONAL WILL BE RESPONSIBLE IN ANY WAY FOR ADDITIONAL COMPENSATION EXCEPT AS PROVIDED IN THE GENERAL REQUIREMENTS SECTION - CHANGES DUE TO SUBSURFACE OR OTHER UNFORESEEN CONDITIONS.
2. MATERIAL TESTING AND SPECIAL INSPECTION AGENCY SHALL INSPECT THE BEARING MEDIUM AND REPORT TO THE DESIGN PROFESSIONAL WHETHER FOUNDATION MEDIUM MEETS THE DESIGN REQUIREMENTS.
3. INDIVIDUAL SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 3,000 PSF AND 3,000 PSF, RESPECTIVELY.

    3.1 NO FOOTINGS SHALL BEAR ON ROCK. UNDERCUT ROCK A MINIMUM OF 2 FEET BELOW BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL.
4. PROOF ROLL BUILDING AREAS WITH TWO COMPLETE COVERAGES OF A LOADED DUMP-TRUCK OR SCRAPER. REPLACE SOFT AREAS WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.
5. DENSIFY BUILDING AREAS AND A MINIMUM OF 15'-0" OUTSIDE THE BUILDING PERIMETER USING A VIBRATORY ROLLER (SEE SPECIFICATIONS).
6. UNDERCUT THE ENTIRE BUILDING AREA TO THE EXTENT SHOWN ON THE STRUCTURAL DOCUMENTS AND REPLACE WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.
7. STRUCTURAL FILL SHALL CONTAIN NO ORGANIC MATERIAL AND BE INSPECTED BY A GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT. STRUCTURAL TESTING/INSPECTION AGENCY SHALL INDICATE TO THE DESIGN PROFESSIONAL IF PROPOSED FILL MEETS THE DESIGN CRITERIA. DESIGN PROFESSIONAL WILL APPROVE OR REJECT THE FILL MATERIAL. STRUCTURAL FILL UNDER SLABS AND WITHIN 10'-0" OF THE BUILDING FOOTPRINT SHALL BE PLACED IN 8-INCH LIFTS OF THICKNESS AND COMPACTED TO AT LEAST 95% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698. THE TOP 12" SUB-BASE UNDER SLABS ON GRADE SHALL BE COMPACTED TO AT LEAST 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY. ALL BACKFILL, COMPACTION AND PROOF ROLLING OPERATIONS SHALL BE OBSERVED BY AN INDEPENDENT TESTING LABORATORY. STRUCTURAL FILL SOIL DENSITY SHALL BE 95 PCF.
8. SLABS-ON-GRADE SHALL BE PLACED ON A 4" GRANULAR BASE, COMPACTED TO 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698, AND COVERED WITH A CONTINUOUSLY SEALED VAPOR BARRIER. SEE ARCHITECT FOR THICKNESS OF VAPOR BARRIER. THE BASE FOR SLABS-ON-GRADE SHALL BE INSPECTED BY THE MATERIAL TESTING AND SPECIAL INSPECTION AGENCY PRIOR TO EACH PLACEMENT OF CONCRETE.

FOUNDATION CONT'D

9. FOOTINGS SHALL BE CENTERED ABOUT COLUMN LINES UNLESS NOTED OTHERWISE.
10. ALL EXTERIOR FOOTINGS AND TURN DOWN SLAB EDGES SHALL PENETRATE TO A MINIMUM DEPTH OF 18" BELOW FINISHED GRADE.

REINFORCEMENT

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND HAVE MINIMUM SIDE AND END LAPS OF 8".
3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.
4. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI 318, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE DESIGN PROFESSIONAL.
5. PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING, UNLESS NOTED OTHERWISE.
6. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:

    6.1 CONCRETE REINFORCEMENT COVER

EXPOSED TO EARTH OR WEATHER:	
• UNFORMED CAST AGAINST EARTH	3" CLEAR
• FORMED #6 AND LARGER	2" CLEAR
• FORMED #5 AND SMALLER	1-1/2" CLEAR
NOT EXPOSED TO EARTH OR WEATHER:	
• SLABS	3/4" CLEAR

    6.2 MASONRY REINFORCING STEEL SHALL BE PLACED IN THE CENTER OF THE WALL UNLESS NOTED OTHERWISE.
7. REINFORCING STEEL DESIGNATED CONTINUOUS SHALL BE LAPPED AS FOLLOWS:
    - CONCRETE REINFORCEMENT: CLASS B TENSION LAP
    - MASONRY REINFORCEMENT: SEE DETAIL 1/S-401
8. ALL DOWELS AND TERMINATING BARS SHALL HAVE A STANDARD 90 DEGREE HOOK.
9. ALL HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL AND/OR CONSTRUCTION JOINTS AND AROUND CORNERS, UNLESS SHOWN OTHERWISE IN DETAILS.

CAST-IN-PLACE CONCRETE

1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS.
2. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:

    2.1 NORMAL WEIGHT STRUCTURAL CONCRETE:

• FOOTINGS	3000 PSI	F0
• SLABS-ON-GRADE	4000 PSI	F0
• EXTERIOR SLABS-ON-GRADE	4500 PSI	F2

3. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE SLAB OR WALL THICKNESS INCLUDING CROSSING UNLESS SPECIFICALLY DETAILED IN THE STRUCTURAL DOCUMENTS. ALL PIPES AND DUCTS SHALL BE PLACED IN THE MIDDLE THIRD OF THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED OTHERWISE IN THE STRUCTURAL DOCUMENTS. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.
4. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.
5. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE DESIGN PROFESSIONAL. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
6. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.016 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE DESIGN PROFESSIONAL.

CONCRETE MASONRY

1. MINIMUM 28-DAY COMPRESSIVE STRENGTH OF ASTM C90 CONCRETE MASONRY UNITS SHALL BE F'M = 2000 PSI.
2. NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY = 2000 PSI.
3. MORTAR SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY AND ASTM C270. MORTAR SHALL BE OF THE FOLLOWING TYPE:
    - WALLS BELOW GRADE: TYPE M
    - BEARING WALLS: TYPE M OR S
4. CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 2500 PSI COARSE GROUT AS SHOWN IN THE STRUCTURAL DOCUMENTS. GROUT FOR REINFORCED AND NONREINFORCED MASONRY SHALL CONFORM TO ASTM C476.
5. PROVIDE HORIZONTAL LADDER-TYPE JOINT REINFORCEMENT WITH NO. 9 GAGE DEFORMED LONGITUDINAL WIRES AT 16" C/C VERTICALLY AND AT 8" BELOW GRADE, UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC. LONGITUDINAL WIRES SHALL BE PLACED IN THE MORTAR JOINTS.
6. PROVIDE OPEN BOTTOM BEAM BLOCK UNITS WITH 3" DEEP MINIMUM WEB OPENINGS AT HORIZONTAL REINFORCEMENT LOCATIONS. A MINIMUM CLEAR SPACE OF ONE BAR DIAMETER SHALL BE PROVIDED BETWEEN THE REINFORCING BARS AND THE FACE OF MASONRY UNITS.
7. PROVIDE CONTROL JOINTS IN ALL CONCRETE MASONRY WALLS AT LOCATIONS APPROVED BY THE DESIGN PROFESSIONAL AT A MAXIMUM SPACING OF 3 TIMES THE WALL HEIGHT OR 25'-0", WHICHEVER IS LESS.
8. SUBMIT WRITTEN CONSTRUCTION PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.
9. MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE #5@32" CENTERED, UNLESS NOTED OTHERWISE.
10. SUBMIT SHOP DRAWINGS FOR MASONRY REINFORCEMENT IN ACCORDANCE WITH SPECIFICATION SECTION 032000.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, UNLESS NOTED OTHERWISE.
    - STRUCTURAL STEEL HSS SHAPES SHALL CONFORM TO ASTM A500, GRADE C.
    - MISCELLANEOUS PLATES AND CONNECTION MATERIAL SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE.
2. BOLTS AND ANCHORS:

    2.1 BOLTED CONNECTIONS SHALL BE TYPE N (BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANE) WITH MINIMUM 3/4" DIAMETER F3125 BOLTS. SUBMIT PROPOSED BOLT TIGHTENING PROCEDURE FOR REVIEW. BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RCSC-2014 (SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS).

    2.2 ANCHOR BOLTS SHALL BE HEADED BOLTS CONFORMING TO ASTM F1554 GRADE 36 AND SHALL BE HEADED RODS OR THREADED RODS WITH HEAVY HEXAGONAL NUT WELDED TO THE BOTTOM OF THE THREADED ROD, GRADE A563A, UNLESS NOTED OTHERWISE.
3. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO BOTH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
4. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DESIGNED FOR THE SERVICE LOAD REACTION VALUES SHOWN ON THE STRUCTURAL DRAWINGS. FOR STEEL MEMBERS WHOSE REACTIONS ARE NOT SHOWN, THE DESIGN REACTION SHALL BE DESIGNED FOR THE SERVICE LOAD REACTION OF 10 KIPS UNLESS SHOWN OTHERWISE ON THE STRUCTURAL DRAWINGS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE DESIGN PROFESSIONAL. **DESIGN PROFESSIONAL SHALL BE COMPENSATED BY THE CONTRACTOR FOR THE COST INVOLVED IN THE REDESIGN OF CONNECTIONS FOR THE CONVENIENCE OF THE CONTRACTOR. STEEL CONNECTIONS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. SHOP DRAWINGS AND CALCULATIONS FOR SUCH CONNECTIONS SHALL BE SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.** REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS. FOR CONNECTION DETAILS DEPICTING ARRANGEMENT CONCEPT OF THE CONNECTION WITHOUT COMPLETE DETAILS, THE CONNECTION DESIGN ENGINEER SHALL FOLLOW THAT ARRANGEMENT CONCEPT IN THE DESIGN. SINGLE ANGLE CONNECTIONS ARE NOT ACCEPTABLE.
5. USE PRE-QUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY D1.1/D1.1M-2015. "NON-PRE-QUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.
8. STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED.

STEEL JOISTS

1. STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED, FABRICATED, AND ERECTED ACCORDING TO THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI).
2. STEEL ROOF JOISTS AND BRIDGING SHALL BE DESIGNED FOR A NET UNIFORM UPLIFT LOAD AS INDICATED ON THE COMPONENTS AND CLADDING DIAGRAM MINUS THE SELF WEIGHT OF THE JOIST AND 5 PSF SUPERIMPOSED DEAD LOAD.
3. BRIDGING SHALL BE DESIGNED TO FULLY BRACE TOP CHORD OF JOISTS UNDER SERVICE LOADS FOR ROOF JOISTS NOT BRACED BY STEEL ROOF DECK.
4. DESIGN OF STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR THE DESIGN OF THE STEEL JOISTS, BRIDGING AND THEIR CONNECTIONS. SUBMIT CALCULATIONS SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.
5. CONTRACTOR SHALL COORDINATE THE CONSTRUCTION AND ERECTION OF WALLS, BEAM FRAMING, METAL DECKING, ETC. TO ENSURE COMPATIBILITY OF ROOF AND WALL SYSTEMS CONSIDERING PITCH AND CAMBER OF STEEL JOISTS.
6. ALL JOIST SEATS SHALL BE CAPABLE OF RESISTING A 2,000 LB SERVICE OVERTURNING FORCE.

METAL DECK

1. DECK DESIGN IS BASED ON THE STEEL DECK INSTITUTE DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS.
2. PROVIDE GALVANIZED ROOF DECK WITH THE FOLLOWING MINIMUM PROPERTIES:

• 1.5 INCH	DEPTH
• 20 GAGE	THICKNESS
• 0.229 IN^3/FT	SECTION MODULUS
• 0.217 IN^4/FT	MOMENT OF INERTIA
• 50,000 PSI	YIELD STRESS

3. DECK IS SPECIFIED BASED ON A THREE SPAN CONDITION. FURNISH HEAVIER GAGE DECK IF REQUIRED FOR ONE OR TWO SPAN CONDITIONS.

METAL BUILDING SYSTEM

1. THE PRE-ENGINEERED BUILDINGS SHOWN ARE SINGLE-SPAN, CONTINUOUS FRAME-TYPE METAL BUILDINGS OF THE NOMINAL LENGTH, WIDTH, EAVE HEIGHT, AND ROOF PITCH INDICATED.
2. SUBMIT COMPLETE STRUCTURAL ANALYSIS AND DESIGN CALCULATIONS, AND FRAME REACTION LOADS FOR THE DESIGN OF FOUNDATIONS.
3. PREPARE SHOP DRAWINGS AND CALCULATIONS UNDER SEAL OF A PROFESSIONAL ENGINEER IN THE STATE OF GEORGIA.
4. CERTIFICATION: SUBMIT WRITTEN CERTIFICATION PREPARED AND SIGNED BY A PROFESSIONAL ENGINEER, REGISTERED TO PRACTICE IN GEORGIA, VERIFYING THAT BUILDING DESIGN MEETS INDICATED LOADING REQUIREMENTS AND CODES OF AUTHORITIES HAVING JURISDICTION.
5. STRUCTURAL FRAMING: DESIGN PRIMARY AND SECONDARY STRUCTURAL MEMBERS AND EXTERIOR COVERING MATERIALS FOR APPLICABLE LOADS AND COMBINATIONS OF LOADS IN ACCORDANCE WITH THE AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE) 7 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES. DESIGN MEMBERS FOR A LATERAL DEFLECTION LIMITATION OF L/240 EVALUATED WITH A 10 YEAR MRI WIND SPEED OF 81 MPH.
6. STRUCTURAL STEEL: FOR DESIGN OF STRUCTURAL STEEL MEMBERS, COMPLY WITH REQUIREMENTS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION'S (AISC) "SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" FOR DESIGN REQUIREMENTS AND ALLOWABLE STRESSES.
7. LIGHT GAUGE STEEL: FOR DESIGN OF LIGHT GAUGE STEEL MEMBERS, COMPLY WITH REQUIREMENTS OF THE AMERICAN IRON AND STEEL INSTITUTE'S (AISI) "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AND "DESIGN OF LIGHT GAUGE STEEL DIAPHRAGMS" FOR DESIGN REQUIREMENTS AND ALLOWABLE STRESSES.
8. WELDED CONNECTIONS: ALL STRUCTURAL WELDED JOINTS SHALL CONFORM TO THE PROVISIONS OF AWS D1.1-10, STRUCTURAL WELDING CODE BY AMERICAN WELDING SOCIETY. THE PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.
9. BOLTED CONNECTION: ALL BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RCSC-2014 (SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS).
10. THE BASE OF THE RIGID FRAME COLUMNS ARE ASSUMED TO BE A PINNED CONNECTION WITH NO BENDING MOMENTS TRANSFERRED TO THE FOUNDATIONS.

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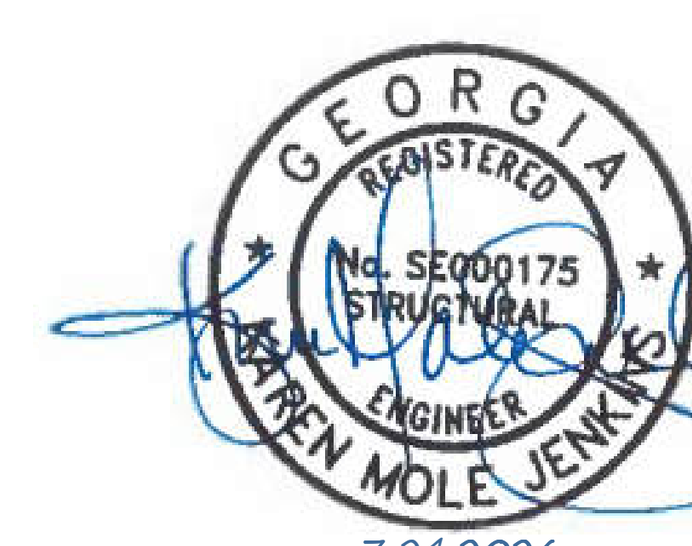
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TCSG-426 LANIER TECHNICAL COLLEGE CDL TRUCK DRIVING FACILITY

2535 LANIER TECH DRIVE
GAINESVILLE, GA 30507



PEF007147
EXP DATE: JUNE 30, 2028

NO.	DATE	DESCRIPTION
5	2026-07-24	DRG BC#2 RESPONSES = STRUCT
4	2026-07-02	DRG BC#2 RESPONSES
2	2026-04-07	DRG BC #1 RESPONSES
0	2026-01-27	CONSTRUCTION DOCUMENTS
ISSUANCE		

PROJECT NUMBER
2025-212

DRAWN: JH
CHECKED: KJ

SHEET TITLE
GENERAL NOTES

SHEET NO.
S-001

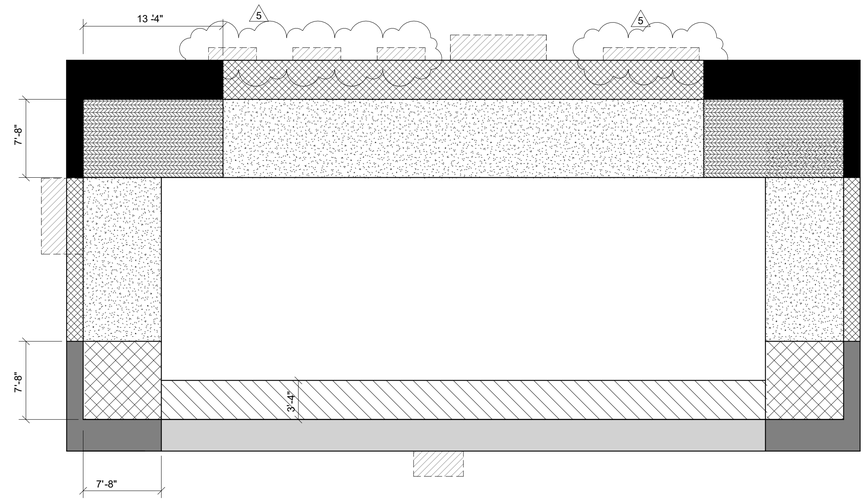
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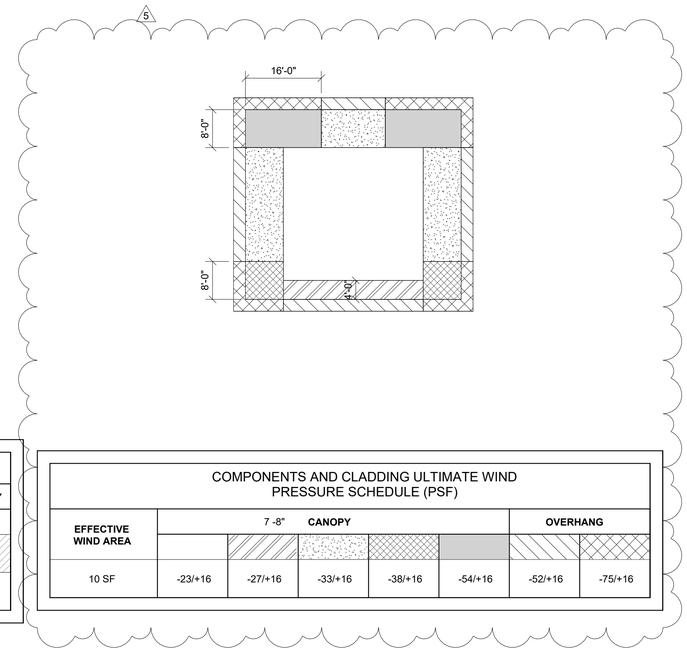
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UPDATED 10/10/23

THIS SHEET PLOTS ON 22" x 34" ANSI D
0 1/2" 1" 2" 3"
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UPDATED 5/1/23





COMPONENTS AND CLADDING ULTIMATE WIND PRESSURE SCHEDULE (PSF)

EFFECTIVE WIND AREA	ROOF					ROOF OVERHANG				WALL		CANOPY
	ZONE 1	ZONE 2	ZONE 3	ZONE 2'	ZONE 3'	ZONE 2	ZONE 3	ZONE 2'	ZONE 3'	ZONE 4	ZONE 5 (NOTE 2)	
25 SF	-26/+16	-30/+16	-36/+16	-36/+16	-49/+16	-45/+16	-55/+16	-51/+16	-68/+16	-23/+21	-27/+21	-16/+16

COMPONENTS AND CLADDING ULTIMATE WIND PRESSURE SCHEDULE (PSF)

EFFECTIVE WIND AREA	7 -8" CANOPY					OVERHANG	
10 SF	-23/+16	-27/+16	-33/+16	-38/+16	-54/+16	-52/+16	-75/+16

SCHEDULE NOTES:
1. (+) AND (-) SIGNS INDICATE PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACE, RESPECTIVELY.
2. PRESSURES APPLY **3'-3"** FROM PROMINENT BUILDING CORNER IN EACH DIRECTION.
3. JOIST MANUFACTURER MAY DEVELOP WIND PRESSURES BASED ON LARGER TRIBUTARY AREAS, DEAD LOAD FOR UPLIFT CALCULATION TO BE 5 PSF.

ABBREVIATIONS

ABT	ABOUT	(E)	EXISTING	IF	INSIDE FACE	OC	ON CENTER
ACI	AMERICAN CONCRETE INSTITUTE	EA	EACH	IBC	INTERNATIONAL BUILDING CODE	OD	OUTSIDE DIAMETER
ADDL	ADDITIONAL	EF	EACH FACE	ICC	INTERNATIONAL CODE COUNCIL	OF	OUTSIDE FACE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	EL	ELEVATION	ID	INSIDE DIAMETER	OPNG	OPENING
ALT	ALTERNATE	ELEC	ELECTRICAL	IE	INVERT ELEVATION	OPP	OPPOSITE
APPROX	APPROXIMATE	ENGR	ENGINEER	IN	INCH	OSH	OVERSIZED HOLE
ARCH	ARCHITECTURAL/ARCHITECT	EOD	EDGE OF DECK	INT	INTERIOR		
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS	EOR	ENGINEER OF RECORD			PCF	POUNDS PER CUBIC FOOT
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS	EOS	EDGE OF SLAB	JT	JOINT	PE	PROFESSIONAL ENGINEER
AWS	AMERICAN WELDING SOCIETY	EQ	EQUAL			PERIM	PERIMETER
		EQUIP	EQUIPMENT	K	KIP(S)	PJF	PREMOLDED JOINT FILLER
B	BOTTOM OF	EW	EACH WAY	KSF	KIPS PER SQUARE FOOT	PL	PLATE
BLDG	BUILDING	EXP	EXPANSION	KSI	KIPS PER SQUARE INCH	PLCS	PLACES
BM	BEAM	EXT	EXTERIOR			PLF	POUNDS PER LINEAR FOOT
BOTT	BOTTOM			(LLH)	LONG LEG HORIZONTAL (ANGLE)	PREFAB	PREFABRICATED
BRG	BEARING	FD	FLOOR DRAIN	(LLV)	LONG LEG VERTICAL (ANGLE)	PSF	POUNDS PER SQUARE FOOT
BTWN	BETWEEN	FDN	FOUNDATION	(LSH)	LONG SIDE HORIZONTAL (HSS)	PSI	POUNDS PER SQUARE INCH
		FF	FINISHED FLOOR	(LSV)	LONG SIDE VERTICAL (HSS)	PT	POINT
C/C	CENTER TO CENTER	FG	FINISHED GRADE	LB	POUND		
CALC	CALCULATION(S)	FIN	FINISH	LF	LINEAR FEET	R	RADIUS
CHKD	CHECKED	FLG	FLANGE	LL	LIVE LOAD	RD	ROOF DRAIN
CIP	CAST-IN-PLACE CONCRETE	FLR	FLOOR	LOC	LOCATION	REF	REFERENCE
CJ	CONSTRUCTION CONTROL JOINT	FRMG	FRAMING	LONG	LONGITUDINAL	REINF	REINFORCING
CJP	COMPLETE JOINT PENETRATION	FRP	FIBER REINFORCED PLASTIC	LP	LOW POINT	REQD	REQUIRED
CL	CENTERLINE	FS	FAR SIDE	LSH	LONG SLOTTED HOLE	RET	RETURN
CLR	CLEAR, CLEARANCE	FT	FOOT	LWC	LIGHT WEIGHT CONCRETE	REV	REVISION
COL	COLUMN	FTG	FOOTING			RO	ROUGH OPENING
CONC	CONCRETE	FV	FIELD VERIFY	M	MOMENT	RTU	ROOFTOP UNIT
CONN	CONNECTION			MAX	MAXIMUM		
COORD	COORDINATE	GA	GAGE, GAUGE	MC	MOMENT CONNECTION	SC	SLIP CRITICAL
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	GALV	GALVANIZED (HOT DIP)	MECH	MECHANICAL	SCHED	SCHEDULE
CTR	CENTER	GRTG	GRATING	MFC	MANUFACTURED	SECT	SECTION
CTRD	CENTERED			MFR	MANUFACTURER	SHT	SHEET
		(H)	HORIZONTAL BEAM ORIENTATION	MIN	MINIMUM	SIM	SIMILAR
DBA	DEFORMED BAR ANCHOR	HCA	HEADED CONCRETE ANCHOR	MISC	MISCELLANEOUS	SL	SLOPE
DBL	DOUBLE	HDR	HEADER	MTD	MOUNTED	SPCS	SPACES
DEG	DEGREES	HGR	HANGER	MTL	METAL	SPEC(S)	SPECIFICATION(S)
DET	DETAIL	HORIZ	HORIZONTAL			SQ	SQUARE
DIA	DIAMETER	HP	HIGH POINT	N&F	NEAR AND FAR	SS	STAINLESS STEEL
DIAG	DIAGONAL	HR	HANDRAIL	N/A	NOT APPLICABLE	SSH	SHORT SLOTTED HOLE
DIR	DIRECTION	HSB	HIGH STRENGTH BOLT	NIC	NOT IN CONTRACT	STD	STANDARD
DL	DEAD LOAD			NO/NO.	NUMBER	STIF	STIFFENER
DWG	DRAWING			NOM	NOMINAL	STL	STEEL
				NS	NEAR SIDE	STRUCT	STRUCTURAL
				NTS	NOT TO SCALE	SYM	SYMMETRICAL
				NWC	NORMAL WEIGHT CONCRETE		

T&B	TOP AND BOTTOM	
T/O	TOP OF	
THK	THINK	
THRU	THROUGH	
TYP	TYPICAL	
UL	UNDERWRITER'S LABORATORIES	
UNO	UNLESS NOTED OTHERWISE	
VERT	VERTICAL	
W	WITH	
W/O	WITHOUT	
WP	WORKING POINT	
WS	WATERSTOP	
WWF	WELDED WIRE FABRIC	

2 S-002 ABBREVIATIONS
3/4" = 1'-0"

STEEL COLUMN/FOOTING TYPE INDICATOR

COL SIZE STEEL COLUMN SIZE
BP-X BASE PLATE MARK
FX.X (-0'-0") FOOTING MARK (T/FTG ELEVATION)
P-X (-0'-0") PEDESTAL MARK (T/PEDESTAL ELEVATION)

FOUNDATION STEP INDICATOR

(-x'-x") T/FOOTING ELEVATION
(-x'-x") T/FOOTING ELEVATION

OPENING IN FLOOR OR ROOF

RECESS/DEPRESSION INDICATOR

STRUCTURAL STEEL CONNECTION AXIAL FORCE

(xxk)

SLOPE INDICATOR

Y
X

CONCRETE SLAB/METAL DECK SPAN INDICATOR

STRUCTURAL STEEL MOMENT CONNECTION

STRUCTURAL STEEL BEAM DESIGNATION

W8x10 (XX) c=x"
(XXk) (XXk)

BEAM SIZE
NUMBER OF STUDS UNIFORMLY SPACED ALONG BEAM
BEAM CAMBER
BEAM SERVICE REACTION

STRUCTURAL STEEL BEAM SPLICE DESIGNATION

CL SPLICE

STRUCTURAL CONCRETE BEAM DESIGNATION

CB XXX PB XXX

CBX INDICATES CONVENTIONALLY REINFORCED CONCRETE BEAM MARK BEAM WIDTH MAY VARY BASED UPON EDGE OF SLAB DIMENSION

PBX INDICATES POST-TENSIONED CONCRETE BEAM MARK, BEAM WIDTH MAY VERY BASED UPON EDGE OF SLAB DIMENSION

1 S-002 STRUCTURAL LEGEND
3/4" = 1'-0"

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LANIER Technical College

TCSG-426 LANIER TECHNICAL COLLEGE CDL TRUCK DRIVING FACILITY

2535 LANIER TECH DRIVE
GAINESVILLE, GA 30507

GEORGIA REGISTERED No. SE000175 PROFESSIONAL ENGINEER KATHRYN MOLE JENKINS
7-24-2026
PEF007147
EXP DATE: JUNE 30, 2028

NO.	DATE	DESCRIPTION
5	2026-07-24	DRG BC#2 RESPONSES = STRUCT
4	2026-07-02	DRG BC#2 RESPONSES
2	2026-04-07	DRG BC #1 RESPONSES
0	2026-01-27	CONSTRUCTION DOCUMENTS
ISSUANCE		

PROJECT NUMBER
2025-212
DRAWN: JH
CHECKED: KJ

SHEET TITLE
ABBREVIATIONS, WIND DIAGRAM & LEGEND

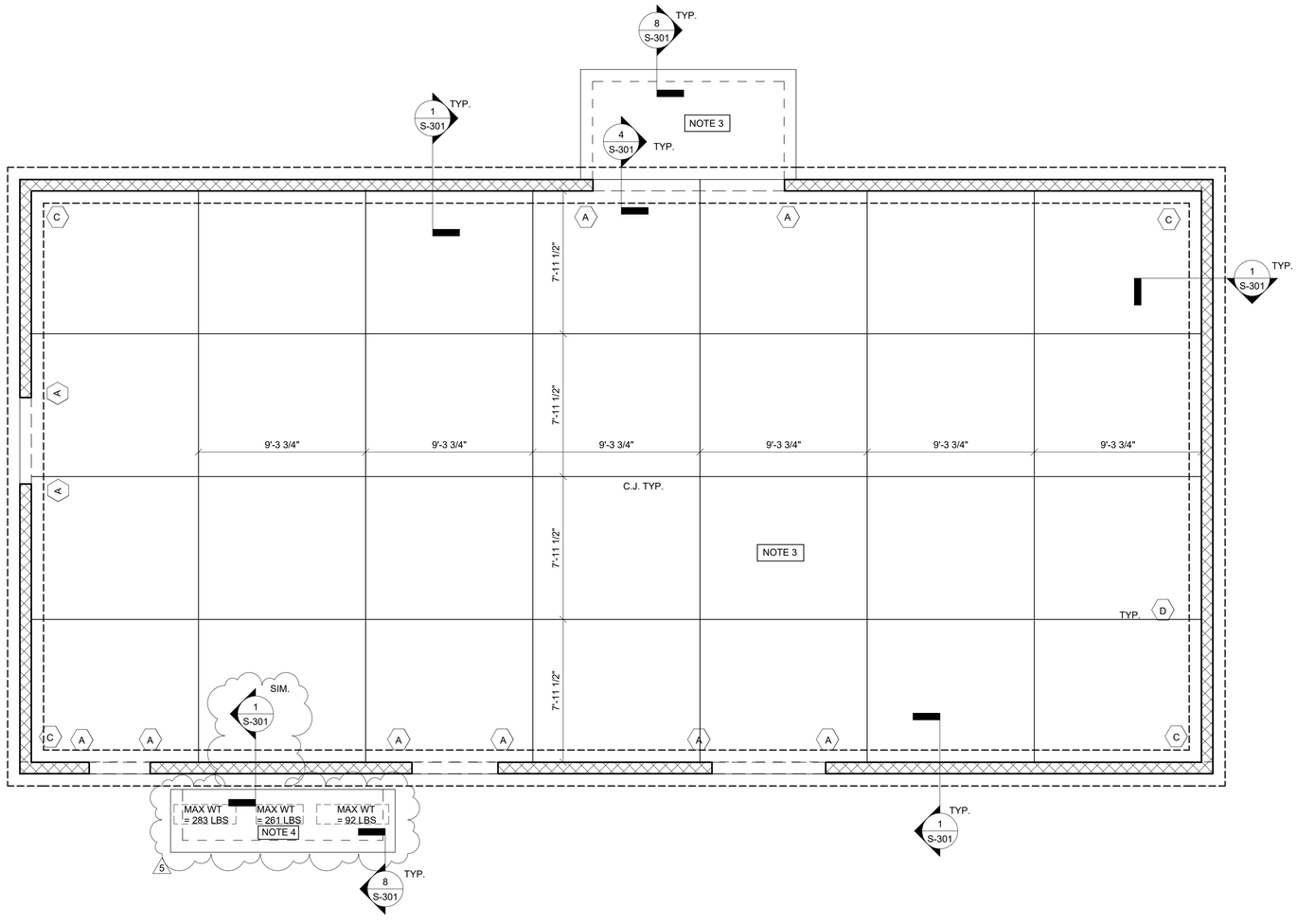
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S-002

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UPDATED
10/17/23



1 S-101 Classroom Foundation
1/4" = 1'-0"

PLAN NORTH

NOTES:
1. SEE S-001 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 4" GRANULAR BASE.
4. PROVIDE 6" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.9xW2.9 ON VAPOR BARRIER AND 4" GRANULAR BASE.
5. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S-301 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
6. ▨ INDICATES MASONRY WALL REINFORCED W/#5@32" O.C. T/FTG = -2'-0". SEE DETAILS, 1,2,3,5 AND 10/S-401.
7. Ⓢ INDICATES MASONRY WALL REINFORCEMENT. SEE DETAIL 5/S-401.
8. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 10/S-301.

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GEORGIA REGISTERED No. SE000175 STRUCTURAL ENGINEER JEREMY MOLE JENKINS
7.24.2026
PEF007147
EXP DATE: JUNE 30, 2028

NO.	DATE	DESCRIPTION
5	2026-07-24	DRG BC#2 RESPONSES = STRUCT
4	2026-07-02	DRG BC#2 RESPONSES
2	2026-04-07	DRG BC #1 RESPONSES
0	2026-01-27	CONSTRUCTION DOCUMENTS
ISSUANCE		

PROJECT NUMBER
2025-212

DRAWN: JH
CHECKED: KJ

SHEET TITLE
FOUNDATION PLAN

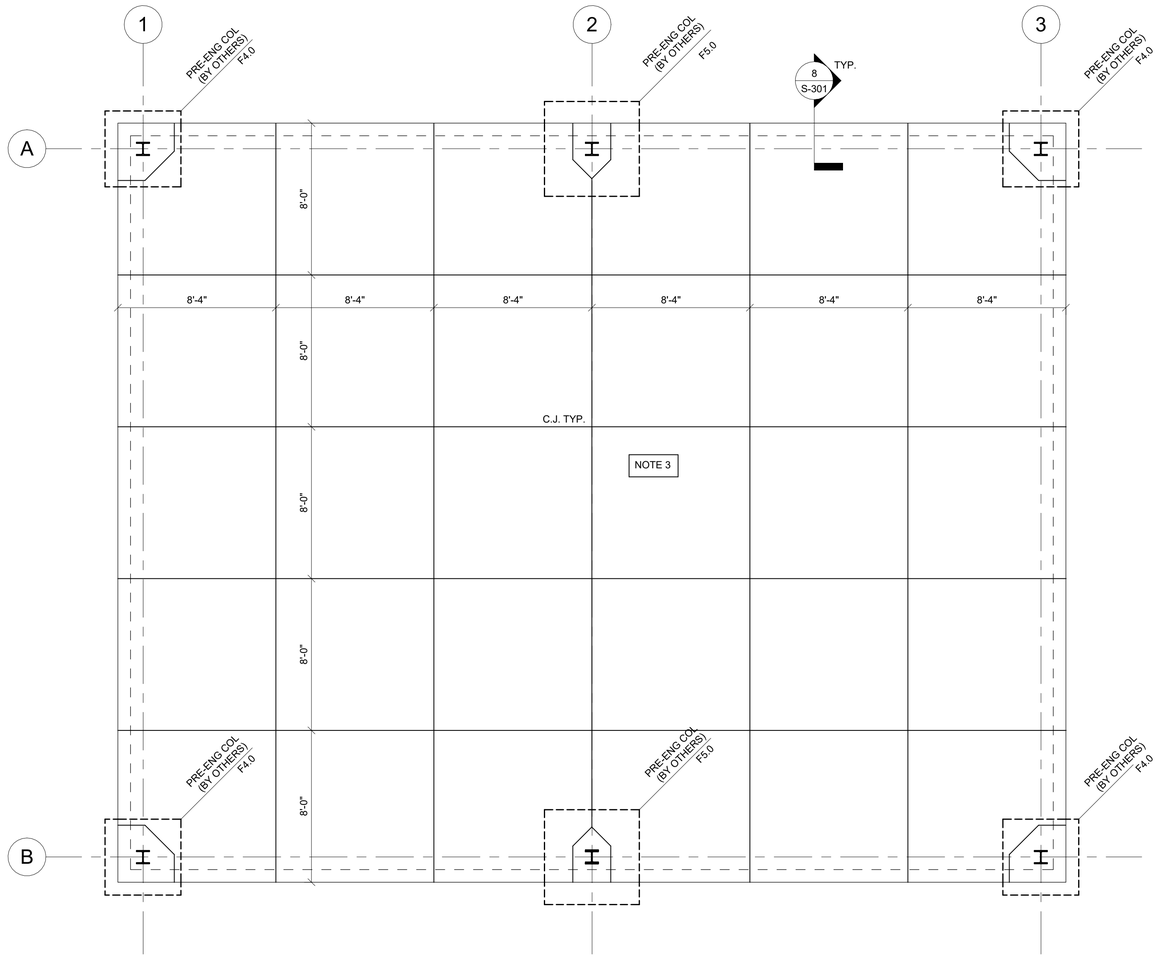
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S-101

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UPDATED 10/1/2025



1 S-102 Pre Engineered Canopy Foundation Plan
1/4" = 1'-0"

PLAN NORTH

NOTES:
1. SEE S-001 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 4" GRANULAR BASE.
4. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S-301 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
5. Fx INDICATED PRE ENGINEERED CANOPY COLUMN FOOTING. SEE 5/S-301 T/FTG = -1'-0" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0".
6. PROVIDE ISOLATION JOINT AT COLUMN. SEE 6/S-301.
7. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 10/S-301.

PRELIMINARY ALLOWABLE DESIGN COLUMN LOADS			
LOCATION	GRAVITY (KIPS)	UPLIFT (KIPS)	THRUST (KIPS)
INTERIOR	25 (KIPS)	4 (KIPS)	5 (KIPS)
END	13 (KIPS)	2 (KIPS)	2.5 (KIPS)

NOTE: LOADS TO BE VERIFIED W/PRE-ENGINEERED CANOPY MANUFACTURER PRIOR TO CONSTRUCTION.

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GEORGIA REGISTERED No. SE000175 STRUCTURAL ENGINEER LAUREN MOLE JENKINS
7.24.2026
PEF007147
EXP DATE: JUNE 30, 2028

NO.	DATE	DESCRIPTION
5	2026-07-24	DRG BC#2 RESPONSES = STRUCT
4	2026-07-02	DRG BC#2 RESPONSES
2	2026-04-07	DRG BC #1 RESPONSES
0	2026-01-27	CONSTRUCTION DOCUMENTS
ISSUANCE		

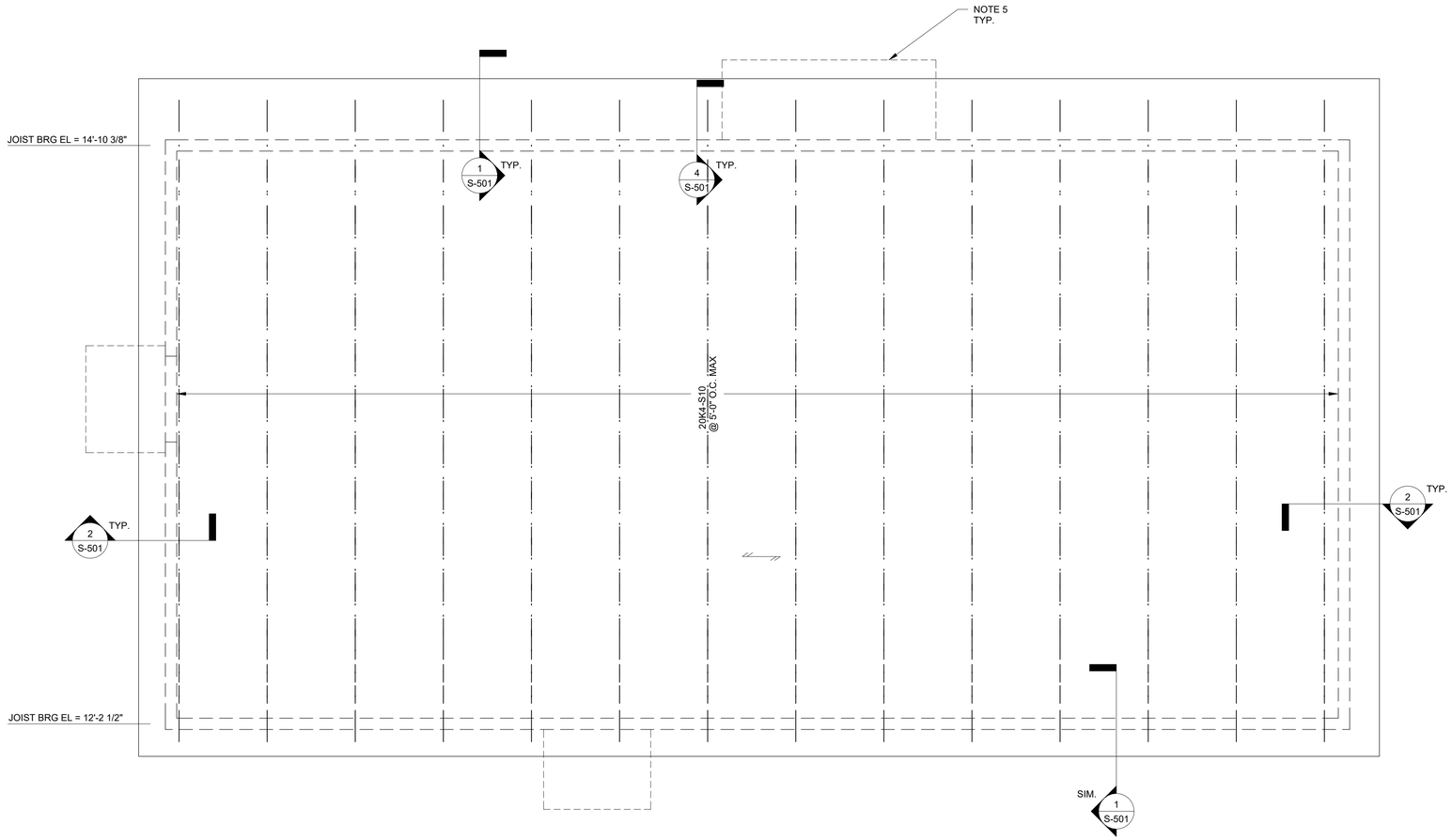
PROJECT NUMBER
2025-212

DRAWN: JH
CHECKED: KJ

SHEET TITLE
PRE-ENGINEERED CANOPY FOUNDATION PLAN

SHEET NO.
S-102

THIS SHEET PLOTS
ON 22" x 34" ANSI D
0 1/2" 1" 2" 3"



1 / S-201 Classroom Roof Framing
1/4" = 1'-0"

PLAN NORTH

NOTES:
1. SEE S-001 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. JOIST BEARING ELEVATION AS INDICATED ON PLAN IS RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" UNO.
4. INDICATES DIRECTIONAL SPAN OF 1 1/2" 20 GA ROOF DECK, SEE 3/S-501
5. INDICATES APPROXIMATE LOCATION OF PREFABRICATED CANOPY. SEE ARCH FOR ELEVATION AND SUPPORT LOCATIONS. CANOPY ARM CONNECTION TO MASONRY BY OTHERS. SEE CONDITION A2 DETAIL 4/S-401 FOR MASONRY REINFORCEMENT AT SUPPORT ARM LOCATIONS.
6. ALL EXPOSED STEEL TO BE GALVANIZED.
7. SEE DETAILS 1, 2, AND 10/S-401 FOR CMU LINTEL SCHEDULE AND DETAILS. COORDINATE ALL OPENING LOCATIONS WITH ARCH.

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GEORGIA REGISTERED No. SE000175 STRUCTURAL ENGINEER KAITREN MOLE-JENKINS
7.24.2026
PEF007147
EXP DATE: JUNE 30, 2028

NO.	DATE	DESCRIPTION
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4	2026-07-02	DRG BC#2 RESPONSES
2	2026-04-07	DRG BC #1 RESPONSES
0	2026-01-27	CONSTRUCTION DOCUMENTS
ISSUANCE		

PROJECT NUMBER
2025-212

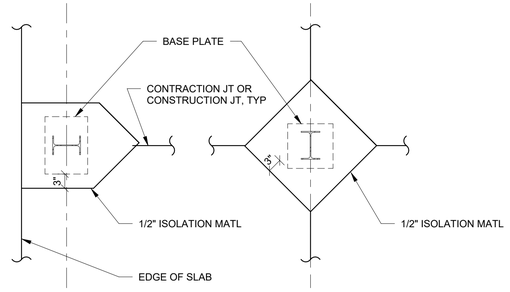
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SHEET TITLE
ROOF FRAMING

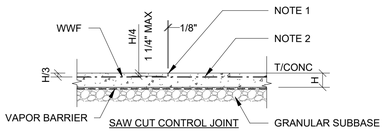
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S-201

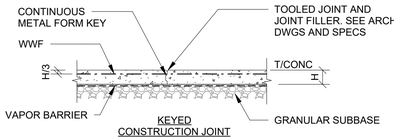
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UPDATED 6/1/023

THIS SHEET PLOTS ON 22" x 34" ANSI D
3" 2" 1" 1/2" 0
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UPDATED 10/17/23



6 S-301 TYPICAL ISOLATION JOINT AT PRE-ENGINEERED COLUMN
3/4" = 1'-0"

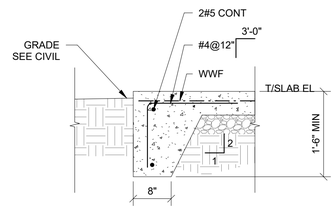




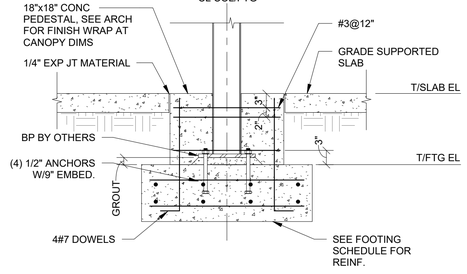
NOTE:
1. SAW CUT SHALL BE MADE SOON ENOUGH TO PREVENT CRACKING, BUT NOT SO SOON AS TO CAUSE SPALLING OF THE CONCRETE WHILE SAWING. FOR JOINT FILLER, SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. FOR JOINT SPACING, SEE STRUCTURAL SPECIFICATIONS.
2. SAW CUT OR DISCONTINUE REINF AT CONSTRUCTION JT.
3. KEYED CONSTRUCTION JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING POURING.
4. WHERE REQUIRED FOR SUCCESSFUL INSTALLATION OF FLOOR FINISH SLAB SHALL BE WET CURED AND BE PLACED A MINIMUM OF 90 DAYS PRIOR TO INSTALLATION OF FLOOR FINISHES. AT CONTRACTOR'S OPTION DIAMOND DOWELS MAY BE USED IN LIEU OF CONTINUOUS CONCRETE KEY AT KEYED JOINT.
5. PROVIDE SLAB BOLSTERS A MAX 2'-0" APART FOR WWF SUPPORT

3 S-301 TYPICAL GRADE SUPPORTED SLAB AT JOINTS
3/4" = 1'-0"

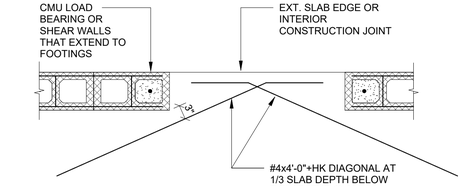
FOOTING SCHEDULE		
MARK	SIZE (WxLxT)	REINF
F4.0	4'-0"x4'-0"x1'-6"	5#6 EW T&B
F5.0	5'-0"x5'-0"x1'-6"	6#6 EW T&B



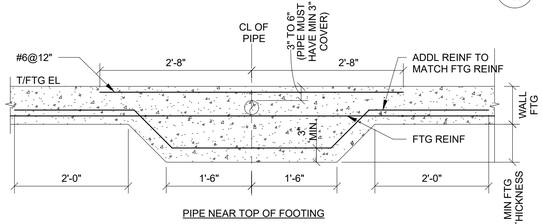
8 S-301 TYPICAL TURNDOWN SLAB
3/4" = 1'-0"

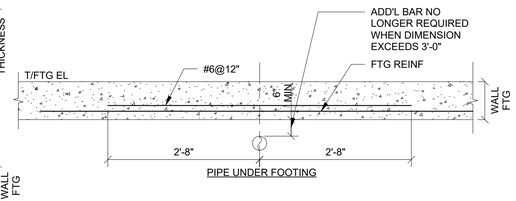


5 S-301 TYPICAL PRE-ENGINEERED COLUMN FOOTING
3/4" = 1'-0"



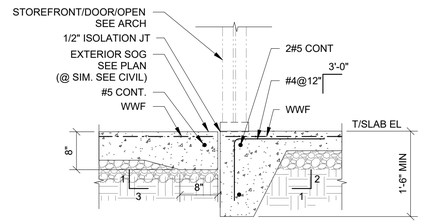
2 S-301 TYPICAL RE-ENTRANT BARS AT CMU WALLS
3/4" = 1'-0"



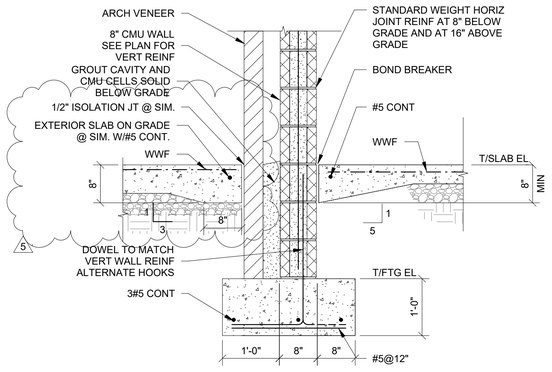


NOTES:
1. NO PIPES TO PASS IN OR BENEATH COLUMN FOOTINGS.
2. PIPES SHALL NOT EXCEED 1/3 THE FOOTING THICKNESS AFTER IT HAS BEEN INCREASED.
3. VERIFY PIPE PENETRATION LOCATIONS W/ PLUMBING CONTRACTOR.

10 S-301 TYPICAL THICKENED FOOTING AT UNDERGROUND PIPING
3/4" = 1'-0"



4 S-301 TYPICAL TURNDOWN SLAB
3/4" = 1'-0"



1 S-301 TYPICAL EXTERIOR 8" CMU WALL FOOTING
3/4" = 1'-0"

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2535 LANIER TECH DRIVE
GAINESVILLE, GA 30507

GEORGIA REGISTERED No. SE000175 STRUCTURAL ENGINEER LAUREN MOLE JENKINS
7.24.2026
PEF007147
EXP DATE: JUNE 30, 2028

NO.	DATE	DESCRIPTION
5	2026-07-24	DRG BC#2 RESPONSES = STRUCT
4	2026-07-02	DRG BC#2 RESPONSES
2	2026-04-07	DRG BC #1 RESPONSES
0	2026-01-27	CONSTRUCTION DOCUMENTS
ISSUANCE		

PROJECT NUMBER
2025-212

DRAWN: JH
CHECKED: KJ

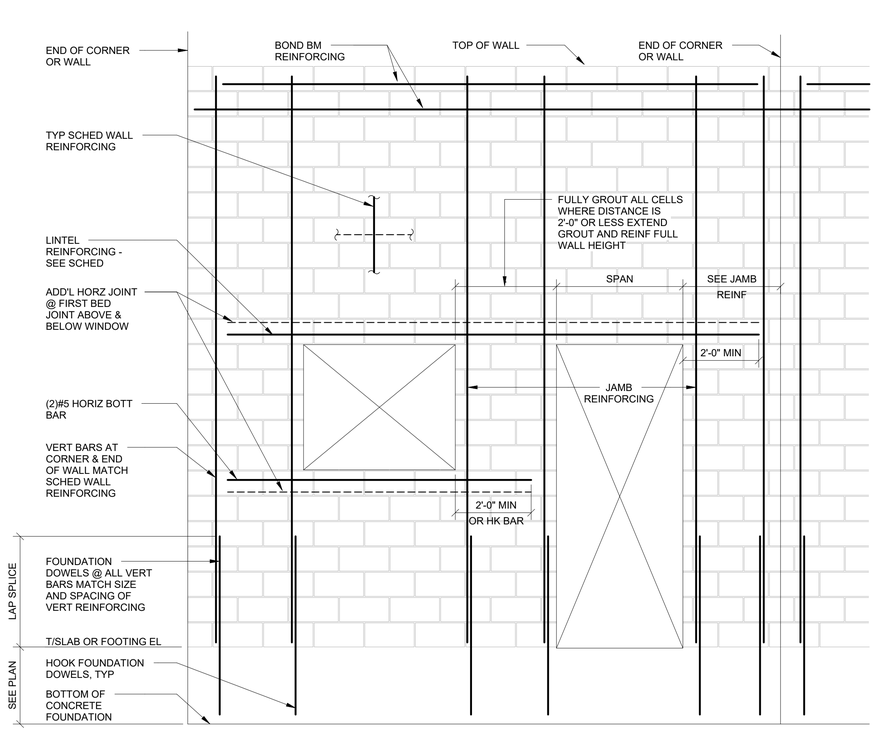
SHEET TITLE
FOUNDATION DETAILS

SHEET NO.
S-301

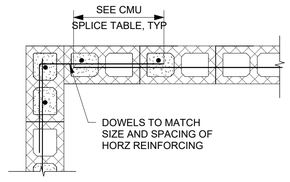
THIS SHEET PLOTS ON 22" x 34" ANSI D
0 1/2" 1" 2" 3"
7/22/2026 8:12:01 PM Autodesk Docs://2025-2012 TCSG-426 LAN CDL Truck Driving Facility/TCSG-426 LAN CDL Range Structural.rvt
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UPDATED 10/17/23

NOTES

1. ALL REINFORCING SHOWN IN ADDITION TO SCHEDULED WALL REINFORCEMENT.
2. ALL REINFORCING SHALL BE SOLID GROUTED USING 2500 PSI (SPEC COURSE GROUT).
3. REFER TO PLANS, GENERAL NOTES, AND WALL ELEVATIONS FOR TYPICAL VERTICAL AND HORIZONTAL WALL REINFORCEMENT.
4. SEE PLAN FOR LOCATION OF CONTROL JOINTS.
5. USE VERT BAR POSITIONERS AT 4 FT.
6. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAM BLOCKS EXCEPT JOINT REINFORCING.
7. HORIZONTAL REINFORCEMENT SHALL BE DISCONTINUOUS THROUGH CONTROL JOINTS EXCEPT BOND BEAMS AT ROOF, FLOORS AND AT MASONRY LINTELS.
8. PROVIDE HORIZONTAL JOINT REINFORCEMENT AT FIRST BED JOINT ABOVE AND BELOW OPENINGS AND EXTEND 1'-0" BEYOND THE OPENING.
9. COORDINATE LOCATION OF SOUND BLOCK IF SPECIFIED WITHIN REINFORCED CMU WALLS WITH ARCHITECTURAL INTERIOR ELEVATION DRAWINGS.



10 S-401 REINFORCEMENT MASONRY WALL ELEVATION
1/2" = 1'-0"



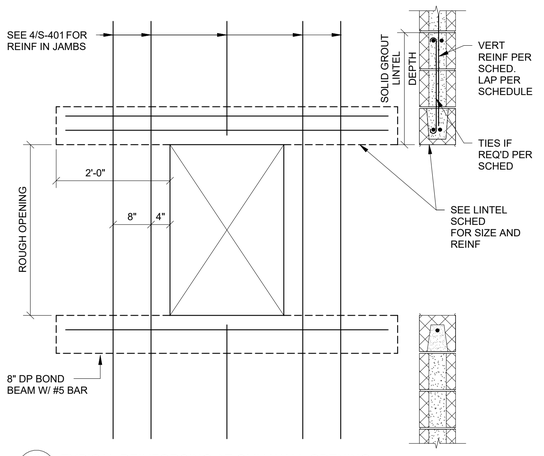
SINGLE LAYER REINFORCING

5 S-401 TYPICAL MASONRY BOND BEAM REINFORCING DETAIL
3/4" = 1'-0"

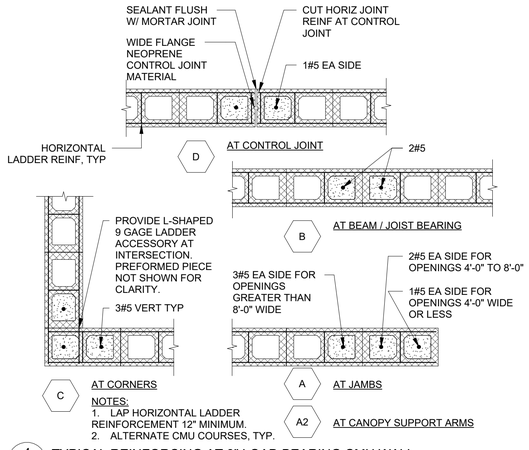
STANDARD LINTEL SCHEDULE		
WALL SIZE	LINTEL TYPE	REMARKS
OPENINGS UP TO 6'-0"		
4" BRICK	5/16"x5"xHORZ LEG BENT PL	ARCH TO PROVIDE HORZ DIM.
8" BLOCK	8"x8" CONC W/ 2#4 T&B 8"x8" U-BLOCK W/ 1#5 T&B	
OPENINGS 6'-0" TO 8'-0"		
4" BRICK	3/8"x6"xHORZ LEG BENT PL	ARCH TO PROVIDE HORZ DIM.
8" BLOCK	8"x8" CONC W/ 2#4 T&B 8"x16" U-BLOCK W/ 1#6 T&B	
OPENINGS 8'-0" TO 10'-0"		
8" BLOCK	8"x8" CONC W/ 2#4 T&B	#3 STIR @ 7" CONT

NOTES:
1. ALL STEEL LINTELS TO BE GALVANIZED.
2. PROVIDE MINIMUM OF 6" BEARING FOR BRICK LINTELS.
3. SEE DETAIL 10/S-401 FOR MINIMUM BEARING OF BLOCK LINTELS.
4. ALL OPENINGS ARE TO BE SHORED UNTIL THE MASONRY HAS CURED FOR A MINIMUM OF 72 HOURS.

3 S-401 STANDARD LINTEL SCHEDULE
12" = 1'-0"

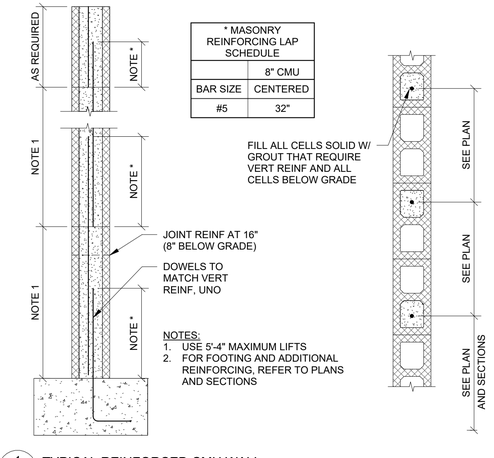


2 S-401 TYPICAL REINFORCING AT CMU WALL OPENING
3/4" = 1'-0"



NOTES:
1. LAP HORIZONTAL LADDER REINFORCEMENT 12" MINIMUM.
2. ALTERNATE CMU COURSES, TYP.

4 S-401 TYPICAL REINFORCING AT 8" LOAD BEARING CMU WALL
3/4" = 1'-0"



* MASONRY REINFORCING LAP SCHEDULE	
	8" CMU
BAR SIZE	CENTERED
#5	32"

NOTES:
1. USE 5'-4" MAXIMUM LIFTS
2. FOR FOOTING AND ADDITIONAL REINFORCING, REFER TO PLANS AND SECTIONS

1 S-401 TYPICAL REINFORCED CMU WALL
3/4" = 1'-0"

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2535 LANIER TECH DRIVE
GAINESVILLE, GA 30507

GEORGIA REGISTERED No. SE000175 STRUCTURAL ENGINEER JEREMY MOLE JENKINS
7.24.2026
PEF007147
EXP DATE: JUNE 30, 2028

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2	2026-04-07	DRG BC #1 RESPONSES
0	2026-01-27	CONSTRUCTION DOCUMENTS
ISSUANCE		

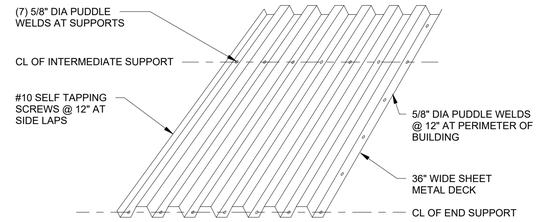
PROJECT NUMBER
2025-212

DRAWN: JH
CHECKED: KJ

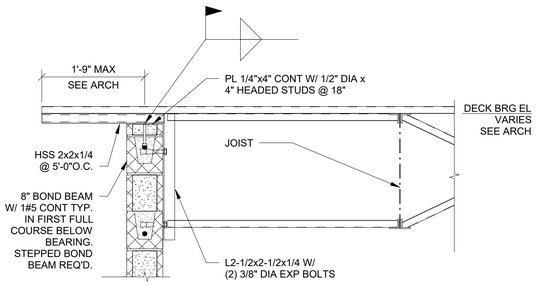
SHEET TITLE
MASONRY DETAILS

SHEET NO.
S-401

THIS SHEET PLOTS ON 22" x 34" ANSI D
0 1/2" 1" 2" 3"
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Autodesk Docs://2025-2012 TCSG-426 LAN CDL Truck Driving Facility/TCSG-426 LAN CDL Range Structural.rvt
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UPDATED 10/10/23

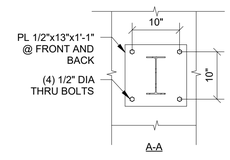


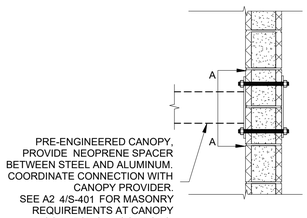
3 S-501 TYPICAL ROOF DECK ATTACHMENT (36" WIDE SHEET)
3/4" = 1'-0"



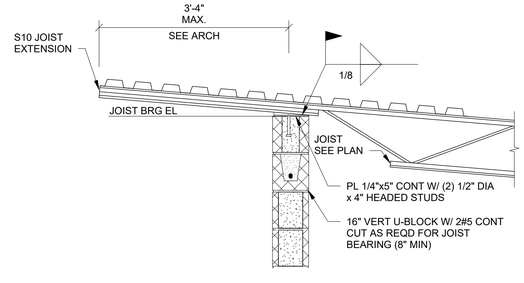
NOTES:
1. VERTICAL REINFORCEMENT NOT SHOWN FOR CLARITY.

2 S-501 TYPICAL 8" CMU WALL AT JOIST
3/4" = 1'-0"





4 S-501 PRE-ENGINEERED CANOPY SECTION AT MASONRY WALL
3/4" = 1'-0"



NOTES:
1. VERTICAL REINFORCEMENT NOT SHOWN FOR CLARITY.

1 S-501 TYPICAL 8" CMU WALL AT JOIST
3/4" = 1'-0"

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0	2026-01-27	CONSTRUCTION DOCUMENTS
ISSUANCE		

PROJECT NUMBER
2025-212
DRAWN: JH
CHECKED: KJ

SHEET TITLE
STEEL FRAMING DETAILS

SHEET NO.
S-501