STRUCTURAL NOTES

CODES AND STANDARDS

1. WIND LOADS AS PER:
   A. INTERNATIONAL BUILDING CODE 2018 W/GEORGIA AMENDMENTS, FOR A 115 MPH(ULT)/90 MPH(ASD) WIND SPEED, EXPOSURE C, 0.18 INTERNAL PRESSURE COEFFICIENT, 1.0 IMPORTANCE FACTOR, AND RISK CATEGORY II.
   B. THIS BUILDING IS DESIGNED AS AN ENCLOSED BUILDING.
2. THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE:
   A. INTERNATIONAL BUILDING CODE 2018 W/GEORGIA AMENDMENTS.
   B. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318/ 2019 EDITION).
   C. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315/ LATEST EDITION).
   D. MANUAL OF STANDARD PRACTICE FOR WELDING REINFORCING STEEL, INSERTS & CONNECTIONS IN REINFORCED CONCRETE CONSTRUCTION. AWS. D1.4/ 2018 EDITION.
3. SNOW LOADS AS PER:
   A. INTERNATIONAL BUILDING CODE 2018 EDITION W/GEORGIA AMENDMENTS.
      B. DESIGN SNOW LOAD:
         1. GROUND SNOW LOAD 22 PSF
         2. FROST LINE = 10"
4. SEISMIC LOADS AS PER:
   A. INTERNATIONAL BUILDING CODE 2018 EDITION W/GEORGIA AMENDMENTS.
      B. SEISMIC DEISGN DATA:
         1. Ss = 0.32 S1 = 0.10
         2. SITE CLASS D
         3. Sds = 0.25g Sd1 = 0.14g
         4. SEISMIC DESIGN CATEGORY C
         5. R = 3.5
         6. CD = 2.25
         7. IE = 1.0
         8. CS = 0.105
         9. EQUIVALENT LATERAL FORCE
         10. V=250K
5. ARCHITECTURAL AND MECHANICAL DRAWINGS:
   A. THE STRUCTURAL DRAWINGS ARE PART OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES PROVIDE ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL CONSULT THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND COORDINATE THE INFORMATION CONTAINED IN THESE DRAWINGS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.
   B. REFER TO ARCHITECTURAL, MECHANICAL OR ELECTRICAL DRAWINGS FOR ADDITIONAL OPENINGS, DEPRESSIONS, FINISHES, INSERTS, BOLTS SETTINGS, DRAINS, REGLETS, ETC.
   C. BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, THE CONTRACTOR SHALL VERIFY ALL MEASUREMENTS TO PROPERLY SIZE OR FIT THE WORK. NO EXTRA CHARGE OR COMPENSATION WILL BE ALLOWED BY THE OWNER RESULTING FROM THE CONTRACTOR'S FAILURE TO COMPLY WITH THIS REQUIREMENT.
   D. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.
   E. ALL STRUCTURES HAVE BEEN DESIGNED TO RESIST THE DESIGN LOADS LISTED ONLY AS COMPLETED STRUCTURES. THE GENERAL CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT WORK IN PROGRESS UNTIL THE STRUCTURES ARE COMPLETED. THE GENERAL CONTRACTOR SHALL ALSO INSURE THAT ITS OPERATIONS AND PROCESURES PROVIDE NO LOADING GREATER THAN THE DESIGN LOADS LISTED ON ANY MEMBER.
6. SECTIONS AND DETAILS:
   ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

SPECIALTY ENGINEERED PRODUCTS

1. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED SHOP DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF GEORGIA. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING.
   A. LIGHT GAUGE METAL INCLUDING, BUT NOT LIMITED TO, SOFFITS, CLADDING, CEILINGS, ETC.
   B. MISCELLANEOUS METALS INCLUDING STEEL STAIRS, MECHANICAL EQUIPMENT SUPPORTS, FRAMES THAT SUPPORT MACHINES, PIPES OR OTHER STRUCTURAL METAL USED FOR SUPPORT OF MECHANICAL SYSTEMS.
   C. MISCELLANEOUS HANGERS, METAL FRAMES, LADDERS, RIGGING, HANGING WALLS, METAL RAILINGS, SAFETY RAILINGS, GLAZING FRAMES, CLADDING SUCH AS STONE, PRECAST, ALUMINUM. METAL PANELS, CABLE BARRIER SYSTEMS, ETC. OR ANY OTHER MISCELLANEOUS PRODUCT REQUIRED BY ANY OF THE CONSTRUCTION DOCUMENTS.

FOUNDATION

1. ALL SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED IN STRICT ACCORDANCE WITH THE:
   A. GEOTECHNICAL REPORT PREPARED BY GEO-HYDRO ENGINEERS, INC, DATED JUNE 23, 2026 PROJECT NO. 261612.20.
2. THE BUILDING SITE SHOULD BE EXCAVATED TO THE DEPTH AND EXTENT INDICATED IN THE SOILS REPORT. ALL SUBGRADES SHALL BE APPROVED IN WRITING BY THE SOILS ENGINEER PRIOR TO BACKFILLING.
3. BOTTOM OF FOOTINGS TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 3000 PSF.
4. TOP OF ALL FOOTINGS SHALL BE MINIMUM 24" BELOW EXTERIOR FINISH GRADE.
5. EXCAVATION & BACKFILL:
   A. ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.
   B. PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.
6. CENTERLINE OF FOOTINGS: SHALL COINCIDE WITH CENTERLINE OF COLUMNS AND WALLS UNLESS OTHERWISE NOTED ON DRAWINGS.
7. DIMENSIONS: ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE VERIFIED AND COORDINATED WITH THE ARCHITECTURAL DRAWINGS BY THE CONTRACTOR BEFORE PROCEEDING WITH THE CONSTRUCTION. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN WRITING BEFORE PROCEEDING WITH ANY WORK.

CONCRETE

1. CONCRETE ELEMENTS TO HAVE THE FOLLOWING STRENGTHS:
   A. FOUNDATIONS 3000 PSI
   B. SLAB-ON-GRADE 3000 PSI
   C. BEAMS/COLUMNS 4000 PSI
   ALL OTHER CONCRETE TO BE 3000 PSI UNLESS NOTED OTHERWISE.
2. ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:
   A. A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI @ 28 DAYS.
   B. SLUMPS SHALL BE 4" MINIMUM AND 6" MAXIMUM.
   C. CONCRETE SHALL HAVE 2 PERCENT AIR ENTRAINMENT.
   D. ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.55.
   E. JOBSITE WATER SHALL NOT BE ADDED.
3. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE ACI BUILDING CODE, THE ACI DETAILING MANUAL (ACI 315/ LATEST EDITION), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301/ LATEST EDITION).
4. SUBMIT ALL REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.
5. CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.
6. WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A 1064, UNLESS OTHERWISE SPECIFIED. PLACE FABRIC 2" CLEAR FROM TOP OF THE SLAB IN SLAB ON GRADE AND SUPPORT ON SLAB BOLSTERS SPACED AT 3'-0" O.C.
7. REQUIREMENTS:
   A. ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A 615 GRADE 60.
   B. WWF SHALL COMPLY WITH ASTM A 1064.
   C. WELDABLE REINFORCING STEEL SHALL COMPLY WITH ASTM A 706.
8. LAP ALL BARS MINIMUM 48 DIAMETERS UNLESS OTHERWISE NOTED ON DRAWINGS. LAP ALL WWF A MINIMUM OF 12 INCHES (UNLESS OTHERWISE NOTED).
9. REINFORCING BARS:
   A. ALL HOOKS SHOWN IN REINFORCEMENT SHALL BE ACI RECOMMENDED HOOKS UNLESS OTHERWISE NOTED.
10. CONSTRUCTION JOINTS IN STRUCTURAL SLABS AND BEAMS SHALL BE AT MID-SPAN AND KEY JOINTED WITH REINFORCING CONTINUOUS ACROSS JOINT AND ADDITIONAL SHEAR FRICTION REINFORCING. CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION. CONSTRUCTION JOINTS IN POST-TENSION SLABS SHALL BE LOCATED AND DESIGNED BY SPECIALTY ENGINEER.
11. ALL MECHANICAL COUPLERS SHALL DEVELOP 1.25 FY OF REBAR IN TENSION OR COMPRESSION AND COMPLY WITH ACI 318.

MASONRY

1. MASONRY UNITS SHALL BE
   A. LOAD BEARING ASTM C 90
   B. TYPE II NON-MOISTURE CONTROLLED
   C. NORMAL WEIGHT
   D. ALL CMU SHALL BE LAID IN A FULL BED OF MORTAR IN RUNNING BOND (U.N.O.).
2. THE COMPRESSIVE STRENGTH OF MASONRY (F'M) SHALL BE 1,500 PSI AS CALCULATED IN ACCORDANCE WITH ASTM C1314.
3. ALL MORTAR SHALL BE IN ACCORDANCE WITH ASTM SPECIFICATION C270
   A. FROM FIELD OBTAINED TEST CUBES. (MIN. OF TWO)
4. GROUT SHALL BE A HIGH SLUMP MIX
   A. IN ACCORDANCE WITH ASTM SPECIFICATION C476
   B. HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI
   C. FROM FIELD OBTAINED TEST CUBES. (MIN. OF TWO)
5. ALL CONCRETE MASONRY BEARING AND SHEAR WALLS SHALL BE
   A. INSPECTED BY A CERTIFIED INSPECTION COMPANY AND CONSTRUCTED IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENT FOR MASONRY STRUCTURES" (TMS 402) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (TMS 602)/ 2016 EDITIONS.
6. PROVIDE 8" X 8" MASONRY BEAM WITH 2 #5 CONT. AT EVERY WINDOW SILL. EXTEND BEAM 8" BEYOND EDGE OF OPENING.
7. PROVIDE HOT DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT (9 GA.) AT 16" ON CENTER VERTICAL IN ALL MASONRY WALLS. PROVIDE DOVE TAIL SLOT ANCHORS AT CONCRETE COLUMNS.
   FOR JOINT REINFORCEMENT, WALL TIES, ANCHORS AND INSERTS, APPLY A MINIMUM COAT OF 1.5 OUNCES PER SQUARE FOOT (PSF) (458/G/M2) COMPLY WITH THE REQUIREMENTS OF ASTM A153, CLASS B.
8. EPOXY GROUT SHALL BE NON-SHRINK HIGH CREEP RESISTANT, AND SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE PROPERTIES:
   TENSILE STRENGTH, ASTM C 30: 1,500 PSI
   FLEXURAL STRENGTH, ASTM C 580: 4,000 PSI
   COMPRESSIVE STRENGTH, ASTM C 579: 1,600 PSI/7 DAYS.
9. MINIMUM LAP SPLICES FOR REINFORCED CMU (WITH F'M = 1,500 PSI):

	BAR SIZE				
CMU SIZE	#4	#5	#6	#7	#8 #9
8"	25"	31"	36"	42"	48" NOTE A

   A. NOT ALLOWED. MAXIMUM BAR DIAMETER SHALL NOT EXCEED ONE-EIGHT OF NOMINAL WALL THICKNESS.
   B. REINFORCING BARS LARGER THAN #9 SHALL BE SPLICED USING MECHANICAL CONNECTORS.
   C. SCHEDULE IS ONLY FOR BARS CENTERED IN THE WALL. ALL EACH FACE REINFORCING BARS SHALL BE SPLICED.

STEEL

1. ALL STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST AISC CODE. STRUCTURAL STEEL SHALL CONFORM TO:
   A. ASTM SPECIFICATION A 992 GRADE 50 FOR ALL WIDE FLANGE BEAMS.
   B. ASTM SPECIFICATION A 36 FOR MISCELLANEOUS STEEL SHAPES (ANGLES, PLATES, ETC.).
   C. SQUARE OR RECTANGULAR HSS SHALL CONFORM TO ASTM SPECIFICATION A 1085 GRADE A (FY=50 KSI).
   D. ALL STEEL TO HAVE A SHOP COAT OF RUST INHIBITIVE PAINT.
   E. DELETE PAINT ON ALL STEEL TO RECEIVE SPRAYED ON FIREPROOFING OR CONCRETE ENCASEMENT.
   F. ALL MILL CAMBER TO BE ORIENTED UPWARD DURING FABRICATION AND ERECTION.
   G. STEEL BEAMS INSTALLED IN PARALLEL WITH STEEL BAR JOISTS MUST HAVE CAMBER EQUAL TO BAR JOISTS.
2. ALL SHOP AND FIELD WELDING SHALL BE PERFORMED BY WELDERS QUALIFIED, AS DESCRIBED IN "AMERICAN WELDING SOCIETY'S STANDARD QUALIFICATION PROCEDURE" (AWS D1.1), TO PERFORM THE TYPE OF WORK REQUIRED.
3. ALL CONNECTIONS SHALL BE BOLTED WITH 3/4" DIAMETER, A-325 HIGH STRENGTH BOLTS OR WELDED (UNLESS SHOWN OTHERWISE ON THE DRAWINGS).
   A. ALL CONNECTIONS TO HOLLOW STRUCTURAL SECTION (HSS) COLUMNS ARE TO BE THRU-PLATE UNLESS NOTED OTHERWISE.
   B. ALL CONNECTIONS SHALL BE DOUBLE ANGLES UNLESS NOTED OTHERWISE.
4. ALL ALUMINUM AND STEEL MEMBERS TO BE TREATED OR PROPERLY SEPARATED TO PREVENT GALVANIC AND CORROSIVE EFFECTS.
5. ALL STEEL WELDING RODS SHALL BE E70XX ELECTRODES.
6. SUBMIT ALL STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.
7. EQUIPMENT SUPPORTS:
   PROVIDE ALL SUPPORTING STEEL NOT INDICATED ON PLAN AS REQUIRED FOR THE INSTALLATION OF MECHANICAL EQUIPMENT AND MATERIALS, INCLUDING ANGLES, CHANNELS, BEAMS, HANGERS, ETC. DO NOT SUPPORT EQUIPMENT OR PIPING FROM METAL DECKING.
8. DECK SUPPORTS:
   PROVIDE 1/4" BENT PLATES AT ALL HIPS, VALLEYS, SKEWED BEAMS AND OTHER AREAS FOR DECK SUPPORT.

JOISTS

1. ALL JOISTS SHALL HAVE A SHOP COAT OF RUST INHIBITIVE NON BITUMINOUS PAINT.
2. JOIST FABRICATOR SHALL HAVE A SPECIALTY ENGINEER REGISTERED IN THE STATE OF GEORGIA SIGN AND SEAL ALL STEEL JOIST SHOP DRAWINGS. THESE SHOP DRAWINGS SHALL CONTAIN A STATEMENT CERTIFYING THAT THE STEEL JOISTS CAN SAFELY RESIST THE WIND UPLIFT FORCES AS NOTED.
3. STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED TO THE REQUIREMENTS OF THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE FOR:
   A. SERIES K JOISTS
   MANUFACTURER SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE. PROVIDE BRIDGING IN ACCORDANCE WITH SJI STANDARDS UNLESS NOTED OTHERWISE ON THE DRAWINGS.
   JOIST SHALL HAVE THE FOLLOWING CAMBERS UNDER THEIR OWN WEIGHTS:
   30' SPAN OR LESS - CAMBER = 0
   30' SPAN TO 60' SPAN - CAMBER = 1" MAXIMUM
   60' SPAN TO 100' SPAN - CAMBER = 1-1/2" MAXIMUM
4. JOIST MANUFACTURER MUST DESIGN BRIDGING ASSUMING NO BRACING IS GIVEN BY THE ROOF DECK. JOIST DESIGN CAN ASSUME THE DECK IS BRACING THE TOPCHORD OF THE JOISTS.

DECK

1. STEEL ROOF DECK SHALL BE:
   A. 1-1/2", 22 GA. TYPE B METAL DECK GALVANIZED AS SHOWN ON ROOF PLAN AS MANUFACTURED BY VULCRAFT/NUCOR OR APPROVED EQUAL. MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE. ROOF DECK MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.
2. ALL ROOF DECK TO BE DESIGNED, MANUFACTURED, AND INSTALLED IN ACCORDANCE WITH LATEST FACTORY MUTUAL STANDARDS.
3. WELDING WASHERS ARE TO BE USED ON ALL CONNECTIONS OF STEEL DECK WITH METAL THICKNESS LESS THAN 22 GA. TO STRUCTURAL STEEL SUPPORTS.
4. IN AREAS OF WARPED ROOF DECK, SELF DRILLING SCREWS ARE TO BE USED ON CONNECTIONS OF STEEL ROOF DECK TO STRUCTURAL STEEL SUPPORTS. SCREW SIZES TO COMPLY WITH MANUFACTURER'S AND FACTORY MUTUAL REQUIREMENTS. ATTACH DECK TO ALL SUPPORTING ROOF JOISTS.
5. 1-1/2" METAL ROOF DECK IS TO BE ATTACHED TO STRUCTURAL STEEL SUPPORTS WITH 5/8" DIAMETER PUDDLE WELDS (MINIMUM OF 5 WELDS PER SHEET PER JOIST). SIDE JOINTS SHALL BE FASTENED TOGETHER WITH #10 SELF DRILLING SCREWS AT MID SPAN BETWEEN SUPPORTS (MINIMUM OF 3 PER SHEET), UNLESS INDICATED OTHERWISE ON THE DRAWINGS.
6. STEEL ROOF AND FLOOR DECK 20 GA OR THINNER SHALL BE GALVANIZED (G90) PER ASTM A653.

SHOP DRAWINGS

1. THE SHOP DRAWINGS SHALL BE SUBMITTED IN COMPLETE PACKAGES FOR THE FOLLOWING:
   A. CONCRETE MIX DESIGNS
   B. CONCRETE REINFORCING STEEL AND WELDED WIRE FABRIC
   C. CONCRETE MASONRY UNIT SUBMITTALS AND OTHER MASONRY ACCESSORIES
   D. SUBGRADE COMPACTION TEST RESULTS(AS THEY RELATE TO THE STRUCTURAL FOUNDATION AND SLAB)
   E. STRUCTURAL STEEL
   F. STEEL JOISTS
   G. METAL DECK
2. PRE-ENGINEERED ITEMS SHALL BE SUBMITTED SIGNED AND SEALED BY A SPECIALTY ENGINEER REGISTERED IN THE STATE OF GEORGIA.
3. CONTRACTOR SHALL REVIEW AND APPROVE ALL SHOP DRAWINGS PRIOR TO SUBMITTING THEM TO ARCHITECT/ENGINEER.

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PROJECT # IP26-38

NO DATE REMARKS

REVISIONS

PHASE I & II – BUILDING DEVELOPMENT

TIMBER DEVELOPMENT CORPORATION

8113 BALL GROUND HWY
BALL GROUND, GA 30107

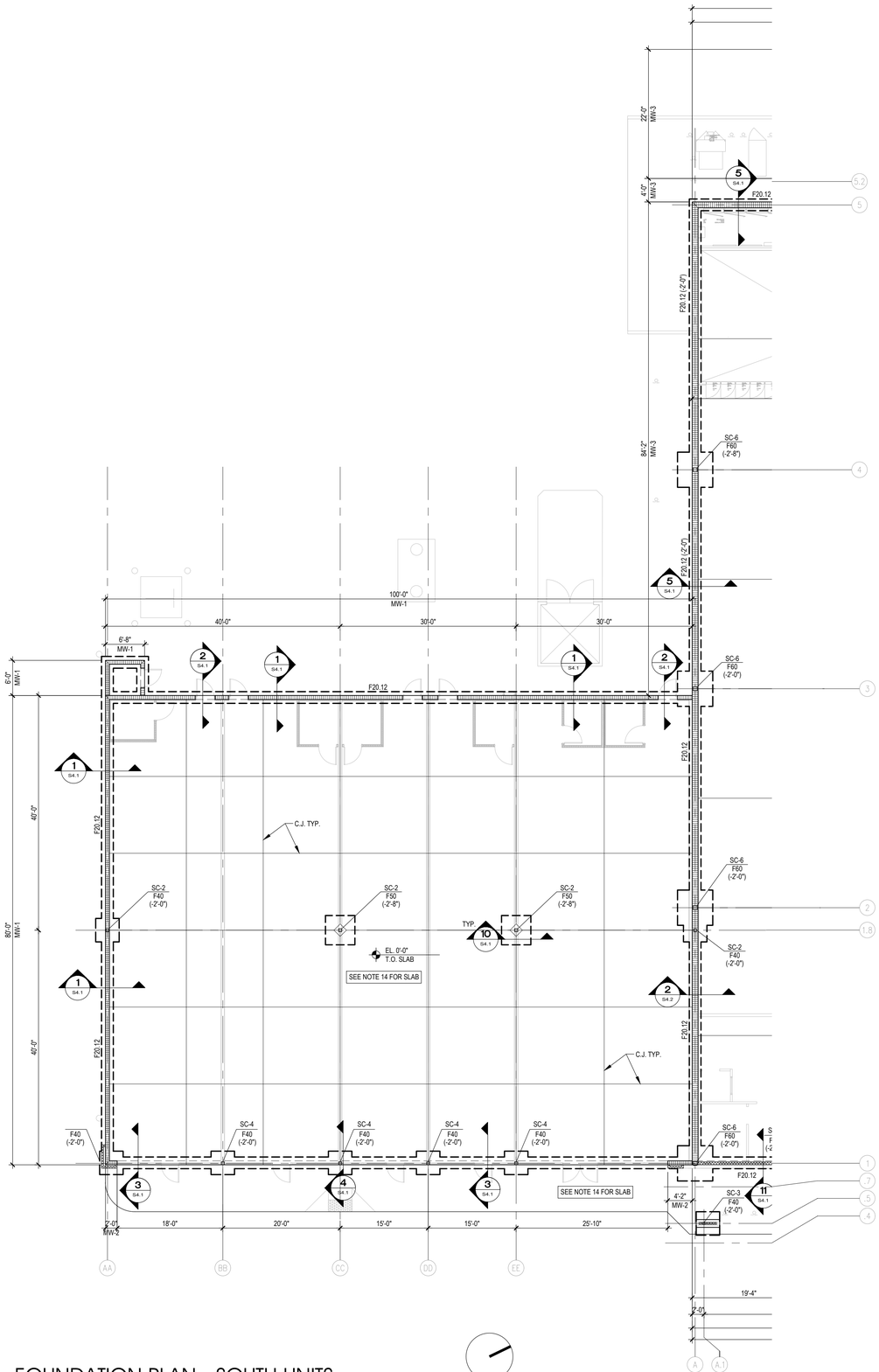
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STRUCTURAL NOTES

CHECKED: BM DRAWN: RED

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FOUNDATION PLAN - SOUTH UNITS

3/32" = 1'-0"

NORTH

FOUNDATION NOTES:

1. ELEV. ±0'-0" IS REFERENCE ONLY. SEE CIVIL FOR TRUE NGVD ELEVATION.
2. S.C. INDICATES SAW CUT SEE S3.1 FOR DETAILS.
3. TOP OF FOOTING ELEVATION = -2'-0" BELOW GRADE.
4. VERIFY SLOPES AND STEPS WITH ARCHI. PRIOR TO CONSTRUCTION. SEE TYPICAL STEP DETAIL ON S3.1.
5. F## DENOTES CONCRETE FOOTING, SEE SCHEDULE ON S3.1.
6. INDICATES FILLED CELL. ALL MASONRY WALLS ARE MW-1 U.N.O., SEE SCHEDULE ON S3.1 FOR SIZE AND SPACING OF REBAR.
7. INDICATES WALL CONTROL JOINT, SEE DETAIL ON S3.1.
8. T.E. DENOTES THICKENED EDGE, SEE DETAIL ON S3.1.
9. SC-# INDICATES STEEL COLUMN SEE SCHEDULE ON S3.1.
10. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR SLAB PENETRATIONS AND DRAIN LOCATIONS.
11. RE-ENTRANT BAR, TYP. AT ALL FLOOR SINKS & DRAINS. NOT ALL LOCATION SHOWN FOR CLARITY. SEE S3.1 FOR MORE INFORMATION.
12. INDICATES METAL STUD WALL, SEE SECTION FOR INFO NOT SHOWN.
13. INDICATES STEP ON FOOTING, SEE TYP. DETAIL ON S3.1.
14. 4" CONCRETE SLAB REINFORCED W/ 6x6-W2.1xW2.1 SET 1 1/2" CLEAR FROM TOP OF SLAB ON 10 MIL MOISTURE RETARDER OVER 4" CRUSHED STONE. SLAB REINFORCING SHALL BE PROPERLY SUPPORTED ON CHAIRS, BOLSTERS, ETC PER SPECIFICATIONS. 6" CRUSHED STONE BASE REQUIRED AT ALL EXTERIOR SLABS.

14. 4" CONCRETE SLAB REINFORCED W/ 6x6-W2.1xW2.1 SET 1 1/2" CLEAR FROM TOP OF SLAB ON 10 MIL MOISTURE RETARDER OVER 4" CRUSHED STONE. SLAB REINFORCING SHALL BE PROPERLY SUPPORTED ON CHAIRS, BOLSTERS, ETC PER SPECIFICATIONS. 6" CRUSHED STONE BASE REQUIRED AT ALL EXTERIOR SLABS.

15. 8" CONCRETE SLAB REINFORCED W/ #4 @ 6" OC EACH WAY SET 1 1/2" CLEAR FROM TOP OF SLAB ON 10 MIL MOISTURE RETARDER OVER 6" CRUSHED STONE. SLAB REINFORCING SHALL BE PROPERLY SUPPORTED ON CHAIRS, BOLSTERS, ETC PER SPECIFICATIONS.
16. 6" CONCRETE SLAB REINFORCED W/ 6x6-W2.1xW2.1 SET 1 1/2" CLEAR FROM TOP OF SLAB ON 10 MIL MOISTURE RETARDER OVER 4" CRUSHED STONE. SLAB REINFORCING SHALL BE PROPERLY SUPPORTED ON CHAIRS, BOLSTERS, ETC PER SPECIFICATIONS. 6" CRUSHED STONE BASE REQUIRED AT ALL EXTERIOR SLABS.
17. CONCRETE CURB AT ELECTRICAL PANEL, REF ARCH AND ELEC FOR EXACT LOCATION AND DIMENSIONS

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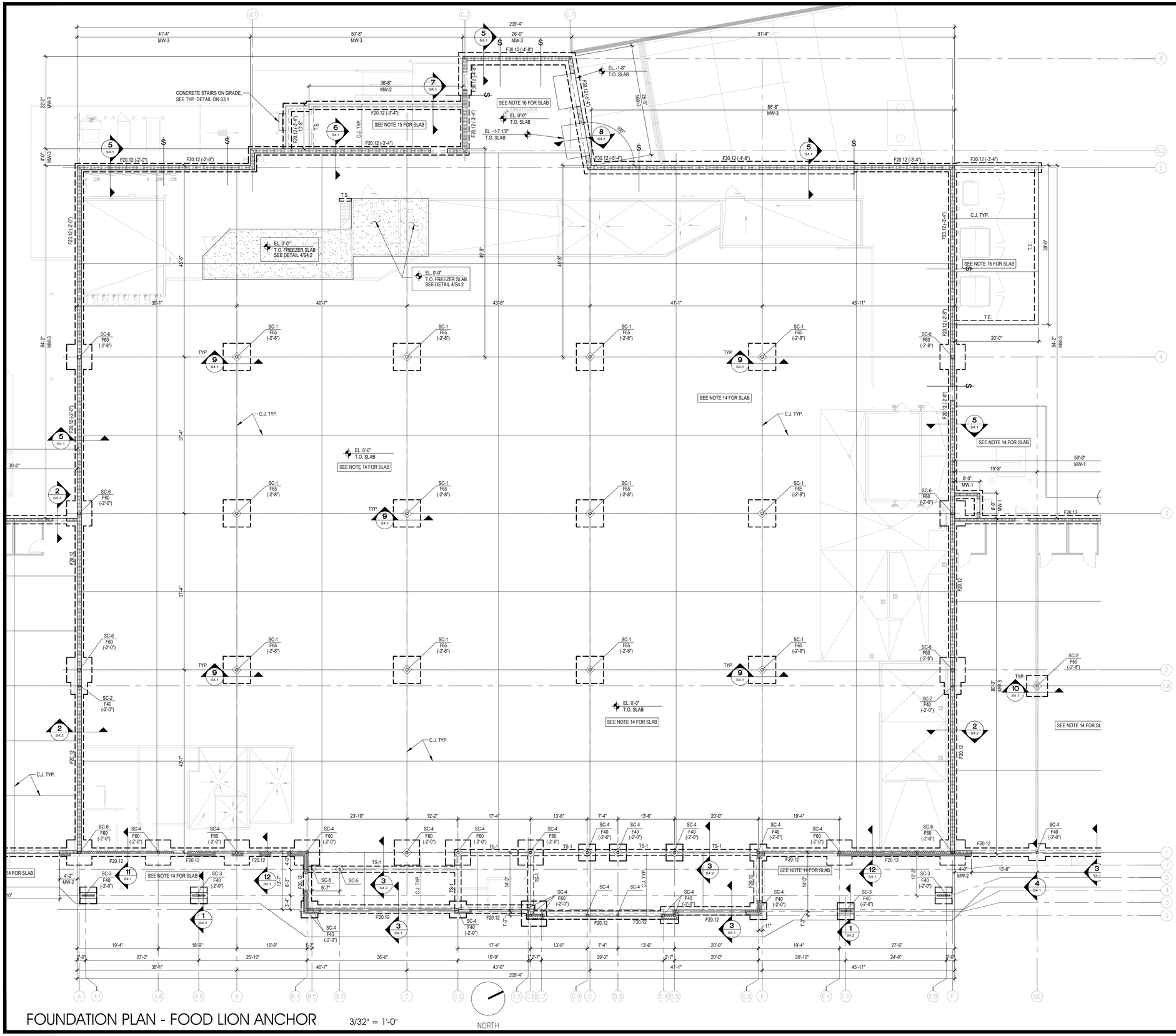
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S1.1

FOUNDATION PLAN
SOUTH UNITS

CHECKED: BM DRAWN: RED

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FOUNDATION PLAN - FOOD LION ANCHOR 3/32" = 1'-0"

FOUNDATION NOTES:

1. ELEV. ±0'-0" IS REFERENCE ONLY. SEE CIVIL FOR TRUE NGVD ELEVATION.
2. S.C. INDICATES SAW CUT SEE S3.1 FOR DETAILS.
3. TOP OF FOOTING ELEVATION = -2'-0" BELOW GRADE.
4. VERIFY SLOPES AND STEPS WITH ARCH'L PRIOR TO CONSTRUCTION. SEE TYPICAL STEP DETAIL ON S3.1.
5. F## DENOTES CONCRETE FOOTING, SEE SCHEDULE ON S3.1.
6. INDICATES FILLED CELL. ALL MASONRY WALLS ARE MW-1 U.N.O., SEE SCHEDULE ON S3.1 FOR SIZE AND SPACING OF REBAR.
7. INDICATES WALL CONTROL JOINT, SEE DETAIL ON S3.1.
8. T.E. DENOTES THICKENED EDGE, SEE DETAIL ON S3.1.
9. SC-# INDICATES STEEL COLUMN SEE SCHEDULE ON S3.1.
10. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR SLAB PENETRATIONS AND DRAIN LOCATIONS.
11. RE-ENTRANT BAR, TYP. AT ALL FLOOR SINKS & DRAINS. NOT ALL LOCATION SHOWN FOR CLARITY. SEE S3.1 FOR MORE INFORMATION.
12. INDICATES METAL STUD WALL, SEE SECTION FOR INFO NOT SHOWN.
13. INDICATES STEP ON FOOTING, SEE TYP. DETAIL ON S3.1.
14. 4" CONCRETE SLAB REINFORCED W/ 6x6-W2.1xW2.1 SET 1 1/2" CLEAR FROM TOP OF SLAB ON 10 MIL MOISTURE RETARDER OVER 4" CRUSHED STONE. SLAB REINFORCING SHALL BE PROPERLY SUPPORTED ON CHAIRS, BOLSTERS, ETC PER SPECIFICATIONS. 6" CRUSHED STONE BASE REQUIRED AT ALL EXTERIOR SLABS.
14. 4" CONCRETE SLAB REINFORCED W/ 6x6-W2.1xW2.1 SET 1 1/2" CLEAR FROM TOP OF SLAB ON 10 MIL MOISTURE RETARDER OVER 4" CRUSHED STONE. SLAB REINFORCING SHALL BE PROPERLY SUPPORTED ON CHAIRS, BOLSTERS, ETC PER SPECIFICATIONS. 6" CRUSHED STONE BASE REQUIRED AT ALL EXTERIOR SLABS.
15. 8" CONCRETE SLAB REINFORCED W/ #4 @ 6" OC EACH WAY SET 1 1/2" CLEAR FROM TOP OF SLAB ON 10 MIL MOISTURE RETARDER OVER 6" CRUSHED STONE. SLAB REINFORCING SHALL BE PROPERLY SUPPORTED ON CHAIRS, BOLSTERS, ETC PER SPECIFICATIONS.
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17. CONCRETE CURB AT ELECTRICAL PANEL, REF ARCH AND ELEC FOR EXACT LOCATION AND DIMENSIONS

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PROJECT # IP26-38

NO DATE REMARKS
REVISIONS

PHASE I & II – BUILDING DEVELOPMENT

TIMBER DEVELOPMENT CORPORATION

8113 BALL GROUND HWY
BALL GROUND, GA 30107

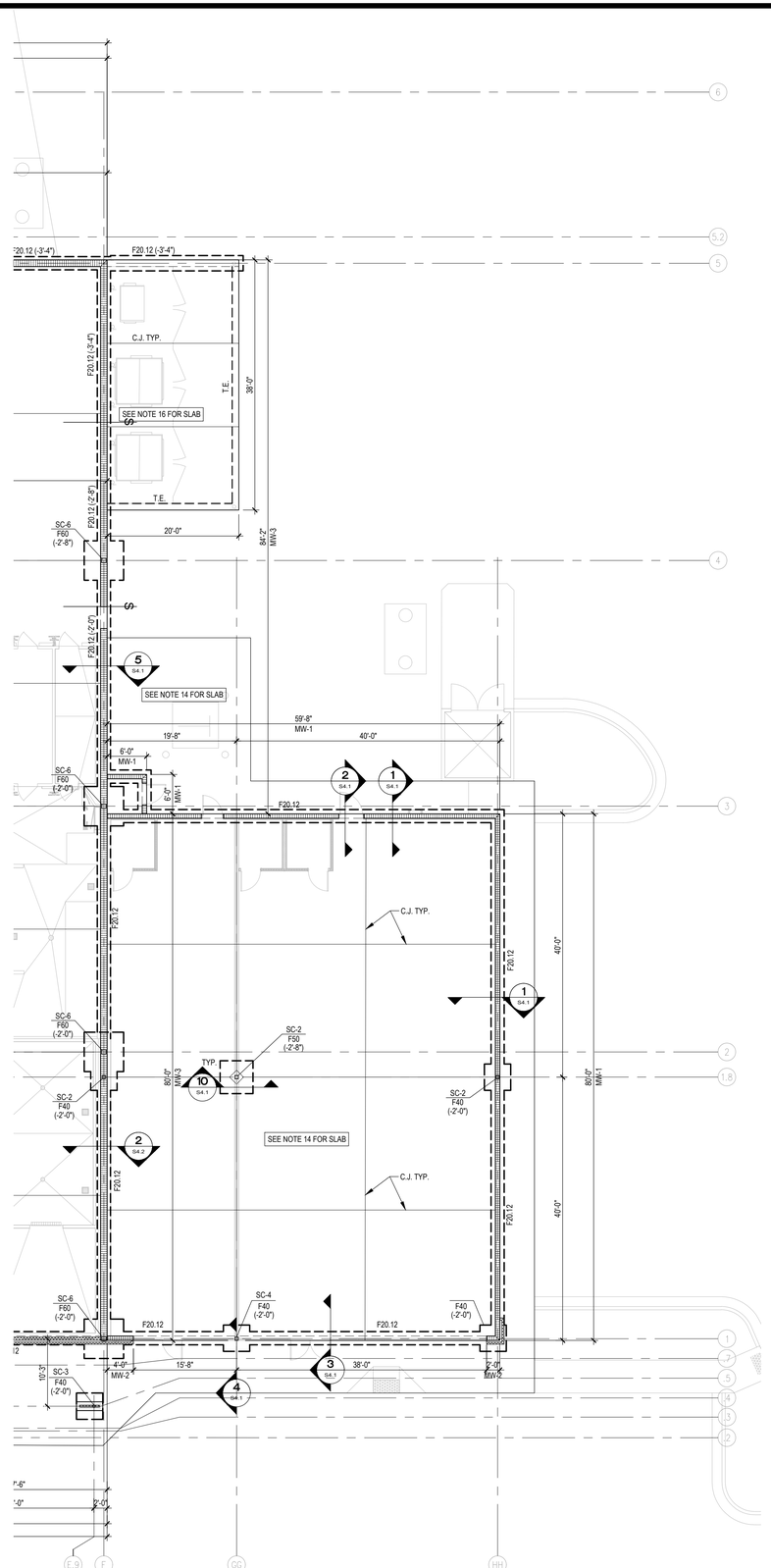
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S1.2

FOUNDATION PLAN
FOOD LION ANCHOR

CHECKED: BM DRAWN: RED

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FOUNDATION NOTES:

1. ELEV. ±0'-0" IS REFERENCE ONLY. SEE CIVIL FOR TRUE NGVD ELEVATION.
2. S.C. INDICATES SAW CUT SEE S3.1 FOR DETAILS.
3. TOP OF FOOTING ELEVATION = -2'-0" BELOW GRADE.
4. VERIFY SLOPES AND STEPS WITH ARCHL PRIOR TO CONSTRUCTION. SEE TYPICAL STEP DETAIL ON S3.1.
5. F## DENOTES CONCRETE FOOTING, SEE SCHEDULE ON S3.1.
6. INDICATES FILLED CELL. ALL MASONRY WALLS ARE MW-1 U.N.O., SEE SCHEDULE ON S3.1 FOR SIZE AND SPACING OF REBAR.
7. INDICATES WALL CONTROL JOINT, SEE DETAIL ON S3.1.
8. T.E. DENOTES THICKENED EDGE, SEE DETAIL ON S3.1.
9. SC-# INDICATES STEEL COLUMN SEE SCHEDULE ON S3.1.
10. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR SLAB PENETRATIONS AND DRAIN LOCATIONS.
11. RE-ENTRANT BAR, TYP. AT ALL FLOOR SINKS & DRAINS. NOT ALL LOCATION SHOWN FOR CLARITY. SEE S3.1 FOR MORE INFORMATION.
12. INDICATES METAL STUD WALL, SEE SECTION FOR INFO NOT SHOWN.
13. INDICATES STEP ON FOOTING, SEE TYP. DETAIL ON S3.1.
14. 4" CONCRETE SLAB REINFORCED W/ 6x6-W2.1xW2.1 SET 1 1/2" CLEAR FROM TOP OF SLAB ON 10 MIL MOISTURE RETARDER OVER 4" CRUSHED STONE. SLAB REINFORCING SHALL BE PROPERLY SUPPORTED ON CHAIRS, BOLSTERS, ETC PER SPECIFICATIONS. 6" CRUSHED STONE BASE REQUIRED AT ALL EXTERIOR SLABS.

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17. CONCRETE CURB AT ELECTRICAL PANEL, REF ARCH AND ELEC FOR EXACT LOCATION AND DIMENSIONS

FOUNDATION PLAN - NORTH UNITS

3/32" = 1'-0"

NORTH

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TIMBER DEVELOPMENT CORPORATION

8113 BALL GROUND HWY
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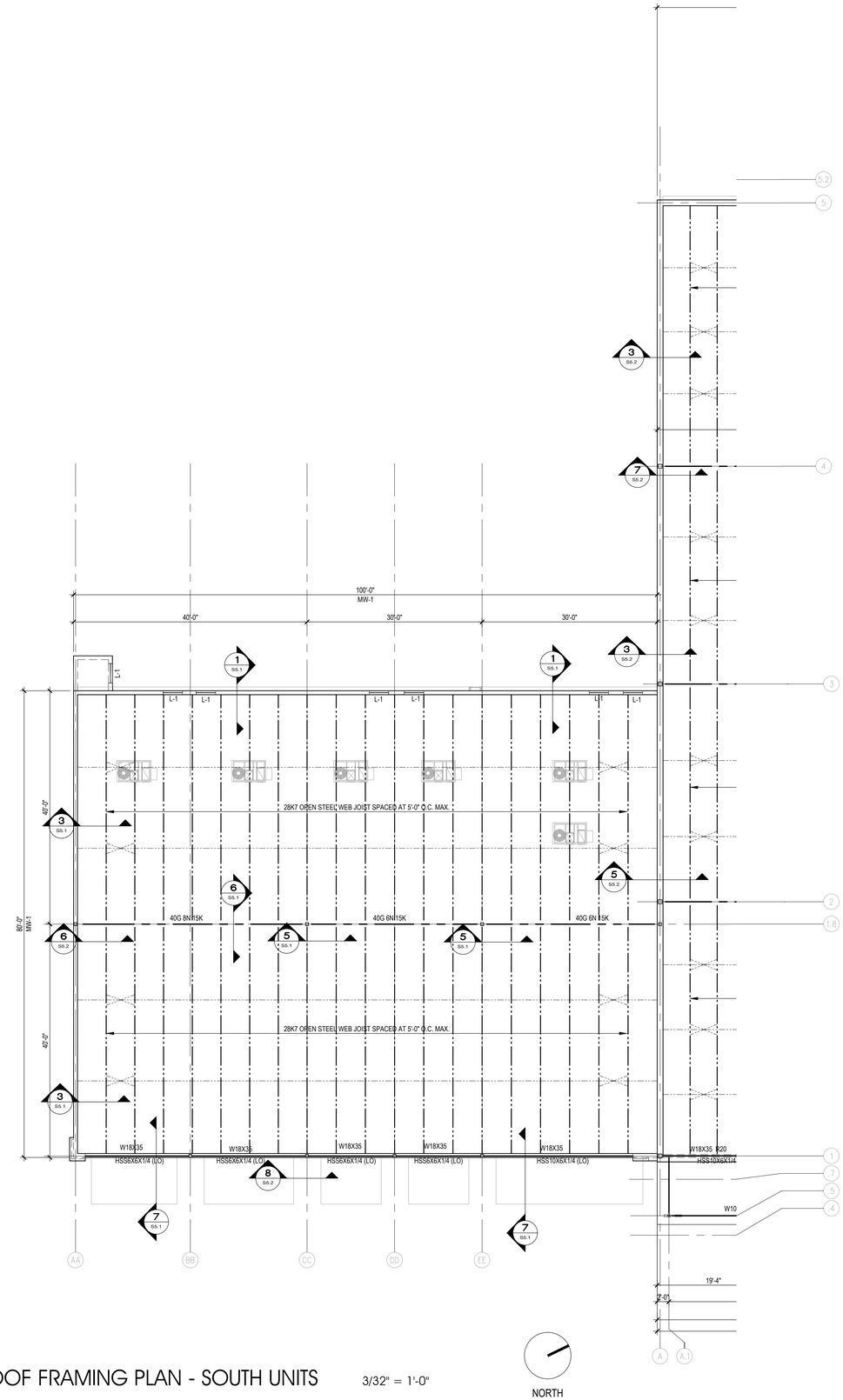
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S1.3

FOUNDATION PLAN
NORTH UNITS

CHECKED: BM DRAWN: RED

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ROOF FRAMING PLAN - SOUTH UNITS

3/32" = 1'-0"

NORTH

ROOF FRAMING NOTES:

1.) SEE PLAN FOR JOIST BEARING ELEVATION.

2.) OPEN WEB STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE SPECIFICATION OF THE STEEL JOIST INSTITUTE. BOTTOM CHORDS FOR K SERIES JOISTS SHALL BE ANGLES OR TEES. SEE STRUCTURAL NOTES FOR BRIDGING REQUIREMENTS. CENTER JOISTS ON COLUMNS AND SPACE A MAXIMUM OF 5'-0" O.C. MAX. U.N.O.

3.) —(1)— ROOF METAL DECK SHALL BE 1 1/2" TYPE B 22 GAGE GALVANIZED. ATTACH DECK TO SUPPORTS WITH 5/8" PUDDLE WELDS WITH USING A 36/5 PATTERN AND AT 6" O.C. AT DECK EDGE. FASTEN SIDE LAPS WITH (3) #10 TEK SCREW BETWEEN EACH SUPPORT.

4.) JOIST BRIDGING SHALL BE PER JOIST MANUFACTURER'S RECOMMENDATIONS.

5.) SEE ARCH FOR DIMENSIONS NOT SHOWN.

6.) VERIFY RTU WEIGHTS AND LOCATIONS WITH MECH. PRIOR TO FABRICATION FRAMING DETAIL.

7.) L- INDICATES POWERS LINTEL, SEE DETAILS ON S4.1.

8.) GROUT SOLID ALL C.M.U. CELLS AT ALL CANOPY ATTACHMENT POINTS.

NOTE: VERIFY W/ ARCH. DRAWINGS ALL TOP OF PARAPET ELEVATIONS AND REPORT DISCREPANCIES TO THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION.

NOTE: VERIFY W/ ARCH. DRAWINGS ALL WALL OPENING SIZES AND LOCATIONS AND REPORT DISCREPANCIES TO THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION.

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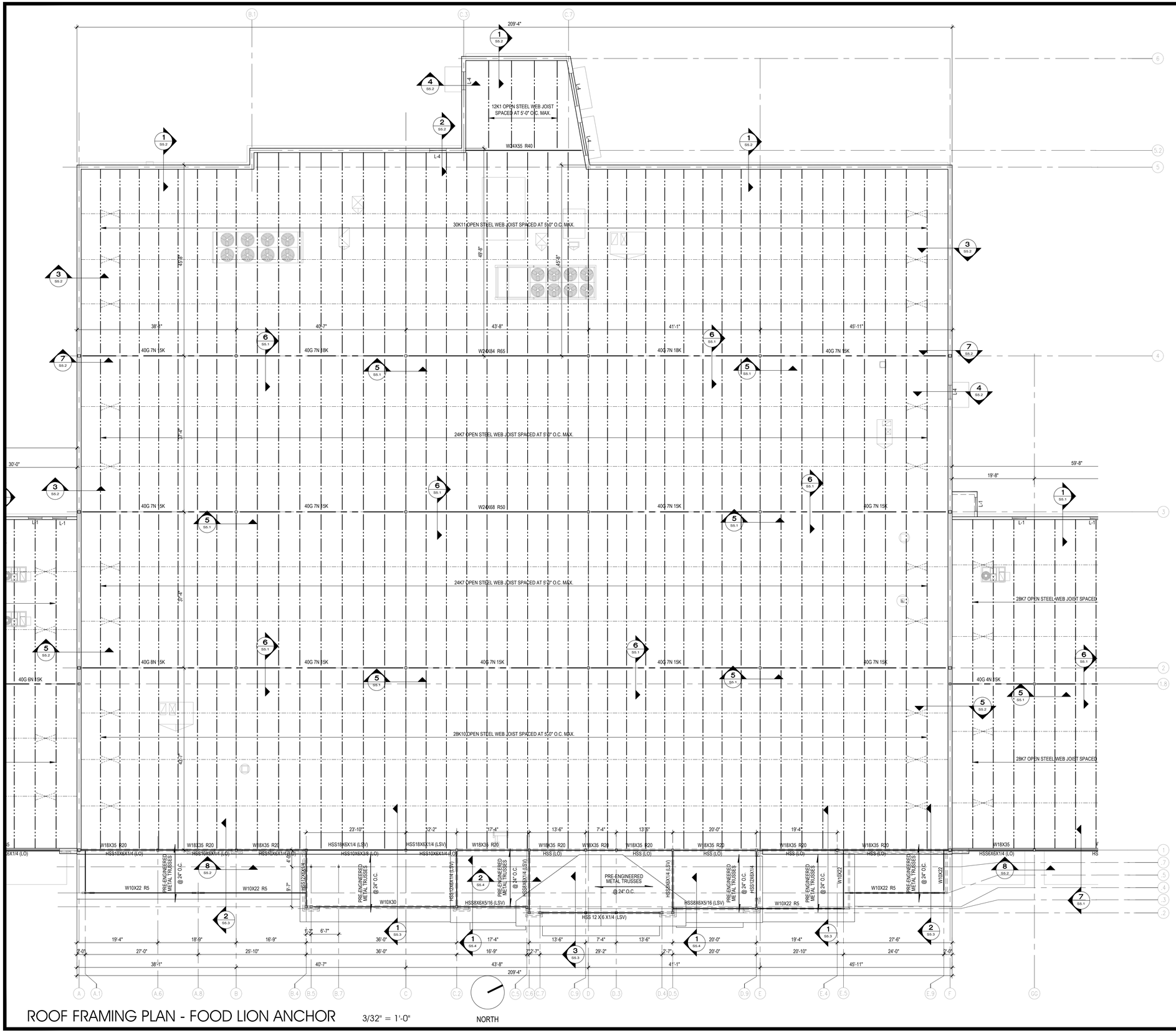
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S2.1

ROOF FRAMING PLAN
SOUTH UNITS

CHECKED: BM DRAWN: RED

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ROOF FRAMING PLAN - FOOD LION ANCHOR

3/32" = 1'-0"

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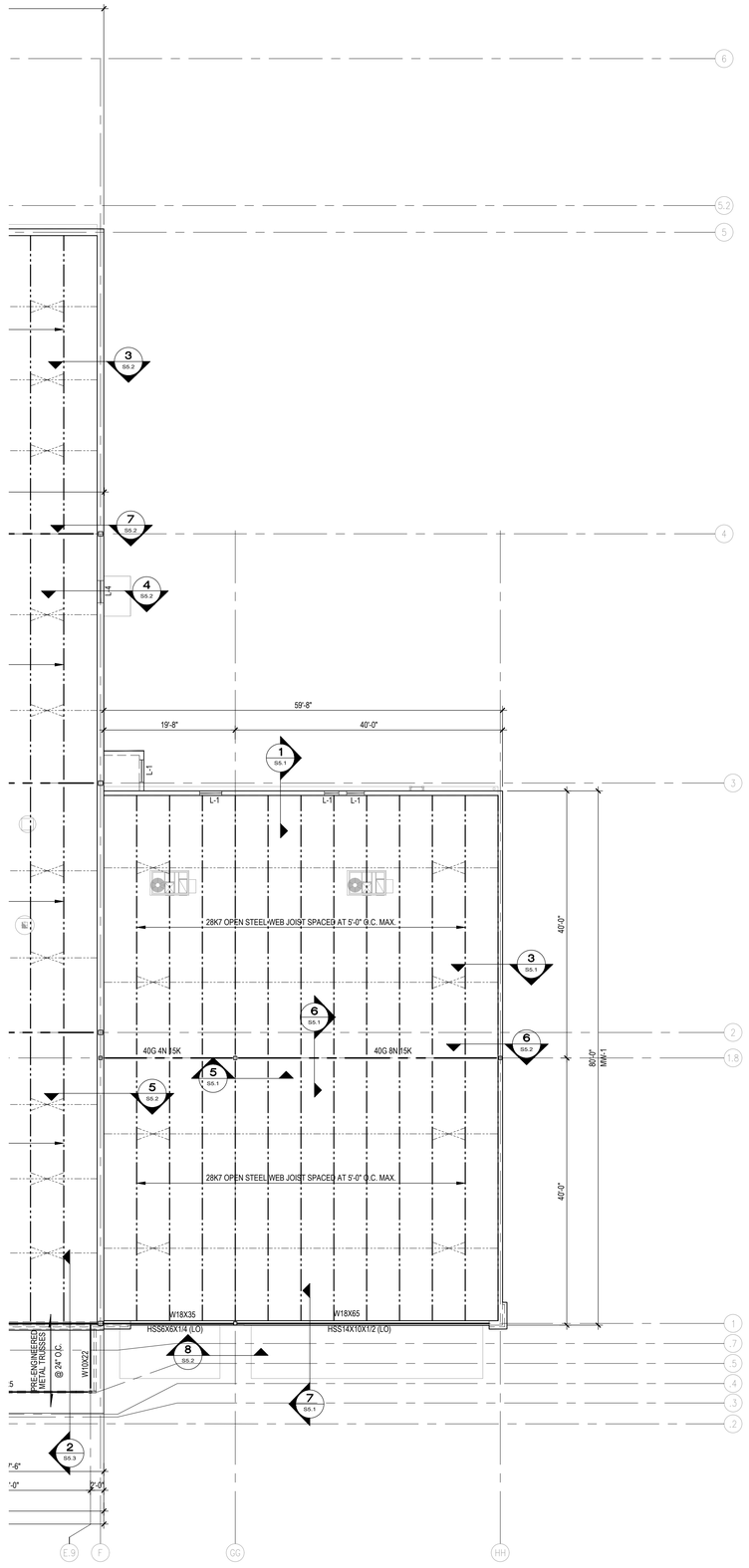
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S2.2

ROOF FRAMING PLAN
FOOD LION ANCHOR

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ROOF FRAMING PLAN - NORTH UNITS

3/32" = 1'-0"

NORTH

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PHASE I & II – BUILDING DEVELOPMENT

TIMBER DEVELOPMENT CORPORATION

8113 BALL GROUND HWY
BALL GROUND, GA 30107

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ROOF FRAMING PLAN
NORTH UNITS

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DESIGN LOAD SCHEDULE
(ALL LOADS SHOWN ARE IN POUNDS PER SQ. FT.)

COMPONENT \ AREA	ROOF
SYSTEM	25
TOTAL DEAD LOAD	25
TOTAL LIVE LOAD	20
TOTAL LOAD	45

RAIN LOAD = 15.6 PSF, RAIN INTENSITY = 4.5 IN/H

STEEL COLUMN SCHEDULE

MARK	SIZE	BASE PL	A.B.	REM
SC-1	HSS 8-1/2 STD PIPE	15x15x1	(4) 3/4"ø	
SC-2	HSS 6x6x1/2	12x12x3/4	(4) 3/4"ø	
SC-3	HSS 4x4x3/8	10x10x3/4	(4) 3/4"ø	
SC-4	HSS 6x6x3/8	12x12x3/4	(4) 3/4"ø	
SC-5	HSS 4x4x1/4	10x10x3/4	(4) 3/4"ø	
SC-6	HSS 8x8x1/4	14x14x3/4	(4) 3/4"ø	

MASONRY WALL SCHEDULE

MARK	THICKNESS	REINFORCING
MW-1	8" CMU	#5 @ 48" O.C.(CENTERED)
MW-2	8" CMU	#5 @ 24" O.C.(CENTERED)
MW-3	12" CMU	#6 @ 48" O.C.(CENTERED)

MASONRY WALL NOTES:

1. WALL SEGMENTS SHALL BE REINFORCED WITH 9 GA. GALVANIZED LATERAL REINFORCING @ 16" O.C. HORIZ. EXTEND REINFORCING 6" INTO POURED ELEMENTS AND AROUND ENCASED STEEL.
2. ADJACENT TO ANY EXTERIOR/INTERIOR WALL OPENING, PLACE 1 #6 VERTICAL IN CELL GROUTED SOLID, FULL HEIGHT.
3. ALL MASONRY REINFORCED CELLS SHALL BE FILLED WITH 3000 PSI GROUT MIX.

FOOTING SCHEDULE

MARK	SIZE	REINFORCING
F20.12	2'-0" X 12" X CONT.	(2) #5 CONT. BOTTOM
F30.12	3'-0" X 12" X CONT.	(3) #5 CONT. BOTTOM
F30	3'-0" X 3'-0" X 12"	(4) #5 E.W. BOTTOM
F40	4'-0" X 4'-0" X 12"	(5) #5 E.W. BOTTOM
F50	5'-0" X 5'-0" X 12"	(6) #5 E.W. BOTTOM
F60	6'-0" X 6'-0" X 14"	(7) #5 E.W. BOTTOM
F65	6'-6" X 6'-6" X 16"	(8) #5 E.W. BOTTOM

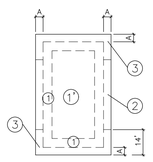
ROOF WIND PRESSURE (PSF)
COMPONENTS AND CLADDING-EXP. C-90 MPH(ASD) WIND SPEED

EFFECTIVE WIND AREA (SQ. FT.)	ROOF AREA 1/1'	2	3
10	+12.9/-28.1	+12.9/-47.1	+12.9/-71.0
20	+12.9/-27.3	+12.9/-42.1	+12.9/-58.7
50	+12.9/-26.4	+12.9/-35.5	+12.9/-42.7
100	+12.9/-25.7	+12.9/-30.4	+12.9/-30.4

IMPORTANCE FACTOR 1.0

NOTES:

1. WIND DESIGN PER IBC-2018
2. +: INDICATES WIND PRESSURE
   -: INDICATES WIND SUCTION
3. ROOF DISTANCE A = 9 FT (COMPONENTS AND CLADDING)
4. FOR EFFECTIVE WIND AREAS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE WIND AREA
5. MULTIPLY VALUES WITH 1.67 FOR ULT.



PLAN

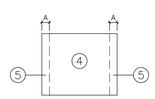
DOOR & WINDOW WIND PRESSURE (PSF)
COMPONENTS AND CLADDING-EXP C-90 MPH(ASD) WIND SPEED

SIZE OF WALL OPENING (SQ. FT.)	WALL AREA 4	5
10	+17.7/-19.1	+17.7/-23.6
20	+16.8/-18.3	+16.8/-22.0
50	+15.7/-17.3	+15.7/-20.0
100	+15.0/-16.5	+15.0/-18.3

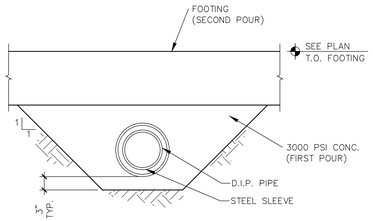
IMPORTANCE FACTOR 1.0

NOTES:

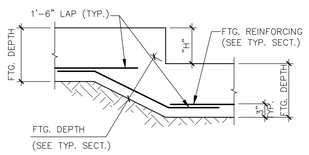
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5. MULTIPLY VALUES WITH 1.67 FOR ULT.



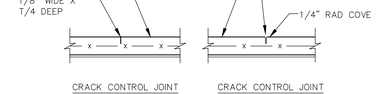
WALL ELEVATION



TYP. PIPE UNDER FTG.



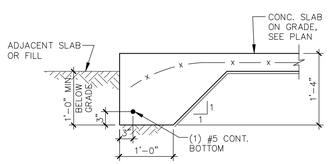
TYP. STEPPED FTG.



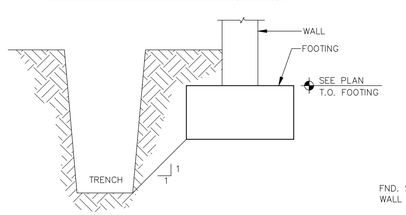
NOTE: THE SLAB SHALL BE SAW-CUT AS SOON AS THE CONCRETE HAS HARDENED SUFFICIENTLY TO WITHSTAND CHIPPING, SPALLING, OR TEARING (WITHIN 8 HRS)

TYPICAL SLAB-ON-GRADE

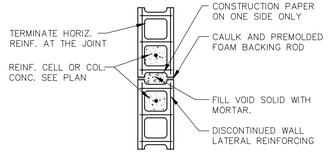
NOTE: CONTROL JOINTS/CONSTRUCTION JOINTS SHALL CREATE PANELS OF 400 SQ. FEET (MAXIMUM)



THICKENED EDGE (T.E.)



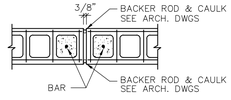
TYP. FTG. ADJACENT TO TRENCH



PLAN VIEW

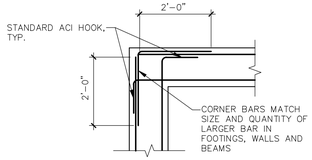
NOTES :

1.- SAW CUT BOND BEAMS, TIE BEAMS 1" DEEP TO CONTINUE WALL CONTROL JOINT TO TOP OF WALL.
2.- CONTROL JOINT SPACING IS NOT TO EXCEED 25'-0"o.c. IN WALLS WITH MORE THAN 25'-0" OF UNINTERRUPTED MASONRY. REFER TO DWG'S. FOR ADDITIONAL SPECIFIED LOCATIONS AS NOTED THUS (WCJ).
3.- CONTINUE ALL BOND BEAMS, TIE BEAMS REINF. THROUGH THE JOINT.

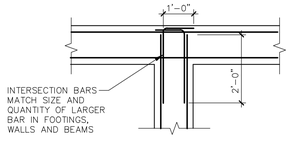


ALTERNATE METHOD

CMU WALL CONTROL JOINT (WCJ) DETAIL
N.T.S.

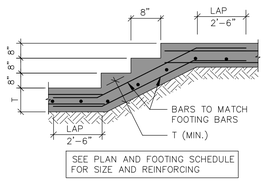


CORNER BAR(S) DETAIL



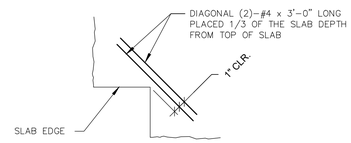
INTERSECTION BAR(S) DETAIL

TYPICAL BAR DETAILS

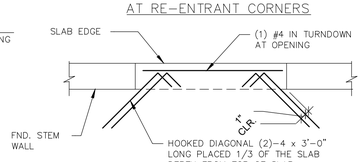


SEE PLAN AND FOOTING SCHEDULE FOR SIZE AND REINFORCING

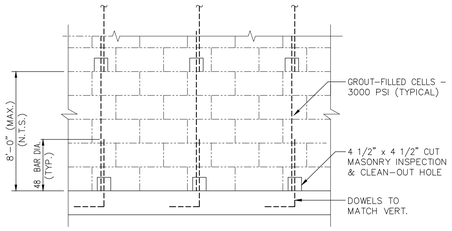
TYPICAL STEP FOOTING DETAIL
N.T.S.



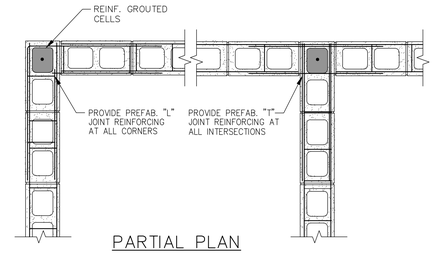
AT RE-ENTRANT CORNERS



AT EXTERIOR OPENINGS

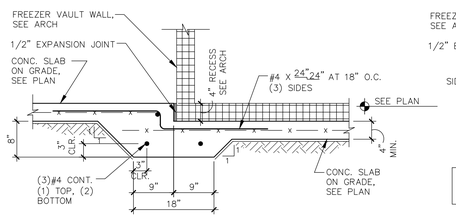


TYPICAL MASONRY FILLED CELL DETAIL
N.T.S.

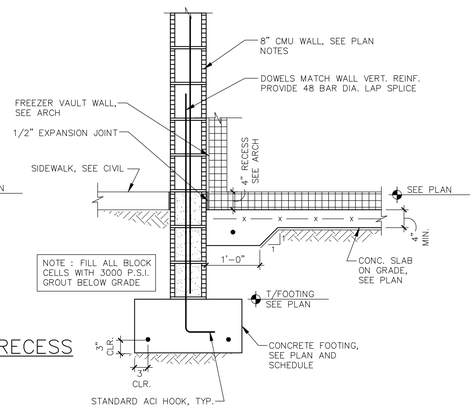


PARTIAL PLAN

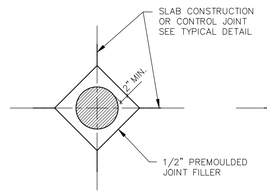
TYPICAL CMU WALL DETAILS
N.T.S.



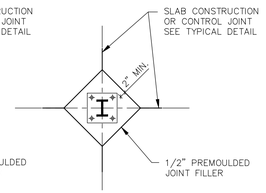
TYPICAL SLAB RECESS



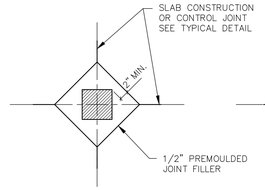
TYPICAL CMU WALL DETAILS
N.T.S.



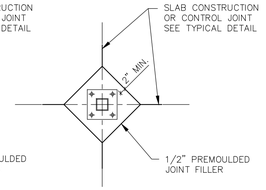
AT ROUND CONCRETE COLUMN



AT STEEL COLUMN

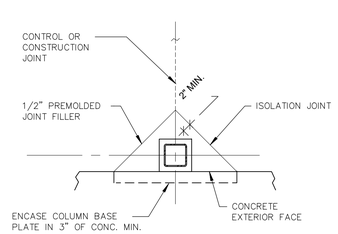


AT CONCRETE COLUMN



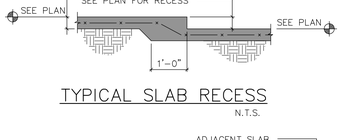
AT STEEL TUBE COLUMN

TYPICAL ISOLATION JOINTS AT COLUMNS
N.T.S.

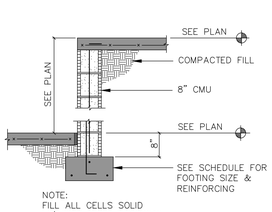


NOTE: JOINT SEALANT BY CONCRETE POLISHER

TYPICAL DIAMOND ISOLATION JOINTS AT COLUMNS
N.T.S.

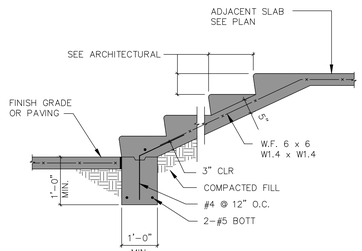


TYPICAL SLAB RECESS
N.T.S.

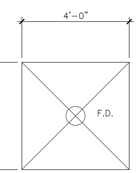


NOTE:
FILL ALL CELLS SOLID
w/ 3000 P.S.I. CONCRETE

TYPICAL CONCRETE PLATFORM
N.T.S.



TYPICAL CONCRETE STAIR ON GRADE
N.T.S.



TYP. EXTENT OF SLOPE SLAB
3/8" = 1'-0"

INTERPLAN
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ARCHITECTURE
ENGINEERING
PERMITTING

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SEAL:

GEORGIA REGISTERED No PE036412 PROFESSIONAL ENGINEER BJARNE MADSEN

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LIC PEF008252
PROJECT # IP26-38

NO DATE REMARKS
REVISIONS

PHASE I & II – BUILDING DEVELOPMENT

TIMBER DEVELOPMENT CORPORATION

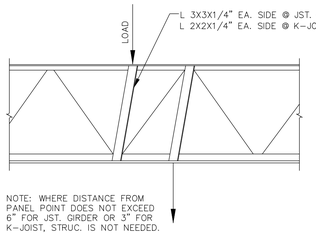
8113 BALL GROUND HWY
BALL GROUND, GA 30107

PROJECT NO: 2026.0771
DATE: 11.02.26

S3.1
STRUCTURAL DETAILS

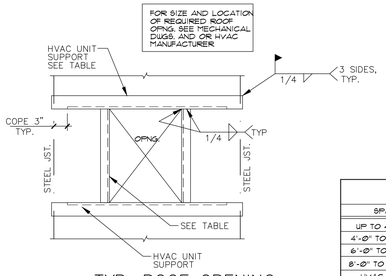
CHECKED: BM DRAWN: RED

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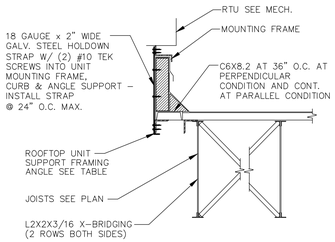
TYP. JST./JST. GIRDER CHORD SUPPORT

FOR LOADS EXCEEDING 300 LBS @ OTHER THAN PANEL POINT LOCATIONS

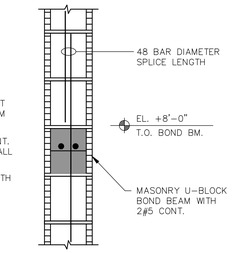


TYP. ROOF OPENING HVAC SUPPORT FMG.

TABLE	
SPAN	L SIZE
UP TO 4'-0"	L3x3x3/16
4'-0" TO 6'-0"	L4x3x1/4 (LLV)
6'-0" TO 8'-0"	L5x3x1/4 (LLV)
8'-0" TO 10'-0"	L6x4x5/16 (LLV)
HVAC UNIT SUPPORT	L6x4x5/16 (LLV)



TYP. RTU HOLDDOWN

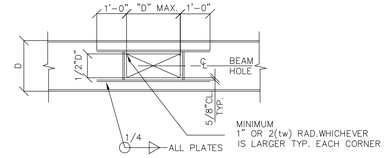


NOTES:

1. WHERE HEIGHT OF MASONRY WALL SEGMENT EXCEEDS 12'-0", INTERMEDIATE BOND BEAM IS REQUIRED.
2. BOND BEAM REINFORCEMENT MUST BE CONT. HOOK BARS INTO REINFORCED JAMB AT WALL OPENINGS.
3. OMIT BOND BEAMS WHERE IT COINCIDES WITH OPENING HEADERS (LINTELS/BEAMS). REINFORCEMENT MUST BE CONTINUOUS AT SUCH LOCATIONS.

TYPICAL BOND BEAM DETAIL

N.T.S.

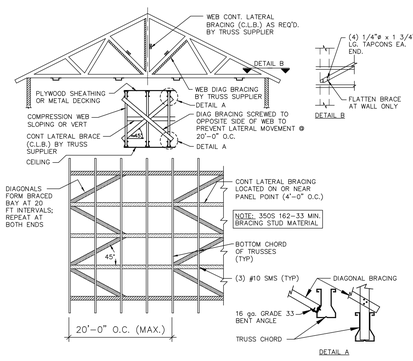


TYPICAL REINFORCEMENT AT OPENING IN STEEL BEAM DETAIL

N.T.S.

NOTES:

1. PROVIDE OPENINGS AS REQUIRED BY MECHANICAL DRAWINGS. OPENINGS MUST NOT BE LOCATED CLOSER TO SUPPORT THAN 1/4 SPAN OF BEAM
2. PROVIDE PLATES AS SHOWN ON BOTH SIDES OF BEAM. THE WIDTH OF ALL PLATES SHALL BE 1/2" THE BEAM FLANGE MINUS 1/2" THE THICKNESS OF ALL PLATES SHALL BE 1/2" MIN. OR THE THICKNESS OF BEAM WEB WHICH EVER IS GREATER.



METAL TRUSSES SHALL BE STORED, HANDLED AND BRACED IN ACCORDANCE WITH THE L.G.S.E.A.'s "FIELD INSTALLATION GUIDE FOR COLD-FORMED STEEL ROOF TRUSSES"

WEB BRACING:

A. THE TRUSS FABRICATOR SHALL PROVIDE AND LOCATE CONTINUOUS LATERAL BRACING (C.L.B.) FOR EACH TRUSS WEB MEMBER AS REQUIRED. ALTERNATE SCAB, "L" AND "T" WEB BRACING MAY BE SUBSTITUTED FOR C.L.B.

B. LATERAL BRACING SHALL BE RESTRAINED BY DIAGONAL BRACING (2" NOMINAL WIDE). THIS BRACING IS TO BE CONTINUOUS.

C. A MINIMUM OF TWO ROWS OF DIAGONAL BRACING IS REQUIRED, ONE AT EACH VERTICAL WEB MEMBER CLOSEST TO BEARING LOCATIONS.

BOTTOM CHORD BRACING:

A. THE BOTTOM CHORDS SHALL BE BRACED BY CONTINUOUS LATERAL BRACING SPACED AT 4'-0" O.C., SCREWED TO BOTTOM OF THE BOTTOM CHORD. DIAGONALS PLACED AT 45° TO THE LATERAL BRACES SHALL BE LOCATED AT EACH END AND AT 20 FOOT INTERVALS. THE MINIMUM BOTTOM CHORD C.L.B. AND DIAGONAL BRACING SHALL BE 350S 162-33, TYPICAL.

B. WHERE THE BOTTOM CHORDS ARE CONTINUOSLY BRACED WITH A RIGID CEILING, C.L.B. SHALL BE SPACED AT 8'-0" O.C. WITH DIAGONAL BRACING.

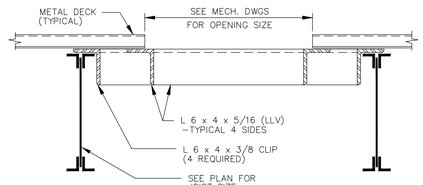
TOP CHORD BRACING:

A. IF PLYWOOD OR METAL DECKING IS APPLIED DIRECTLY TO TOP CHORD, PROPERLY LAPPED AND SCREWED TO DEVELOP DIAPRAGHM ACTION, BRACING IS NOT REQUIRED.

B. IF PURLINS ARE USED, DIAGONAL TOP CHORD BRACING IS REQUIRED AT EACH END. IF BUILDING EXCEEDS 60 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.

METAL TRUSS BRACING DETAIL

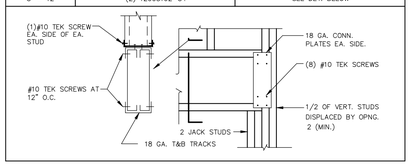
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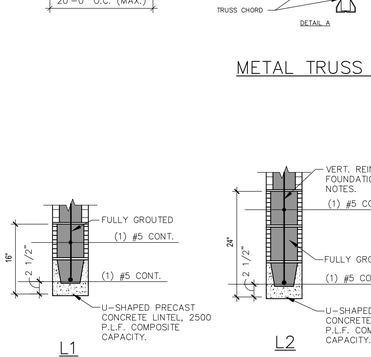
1. PROVIDE AT PERIMETER OF ALL ROOFTOP MECHANICAL UNITS AND OPENINGS AS REQUIRED.
2. SEE MECH. DWGS FOR OPENING SIZE & LOCATION.
3. PROVIDE L 3 x 3 x 5/16 AT ROOF SUMPS

TYPICAL ROOF OPENING DETAIL

NEW WALL METAL STUD HEADER SCHEDULE		
OPNG.	HEADER SIZE	REMARKS
0' - 4'	(2) 800S162-43	SEE DET. BELOW
4' - 8'	(2) 1000S162-43	SEE DET. BELOW
8' - 12'	(2) 1200S162-54	SEE DET. BELOW



TYPICAL DOOR OPENING DETAIL IN NEW NON-LOAD BEARING METAL STUD WALL

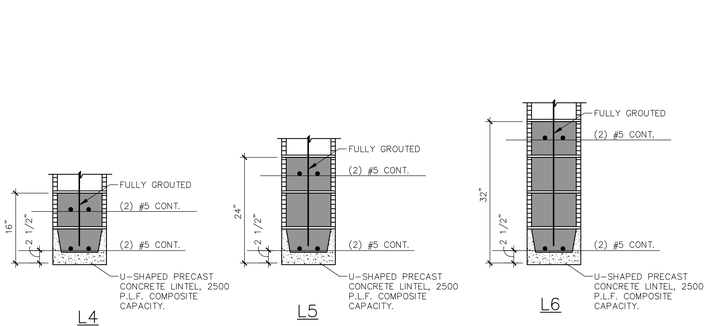


8" C.M.U. PRECAST LINTEL SCHEDULE

N.T.S.

NOTES:

1. SHORE PRECAST LINTEL PER MANUFACTURER RECOMMENDATIONS.
2. VERIFY W/ ARCH'L DWGS. FOR SIZE & LOCATIONS OF OPENINGS.
3. PROVIDE 8" MIN. BEARING @ EACH END.

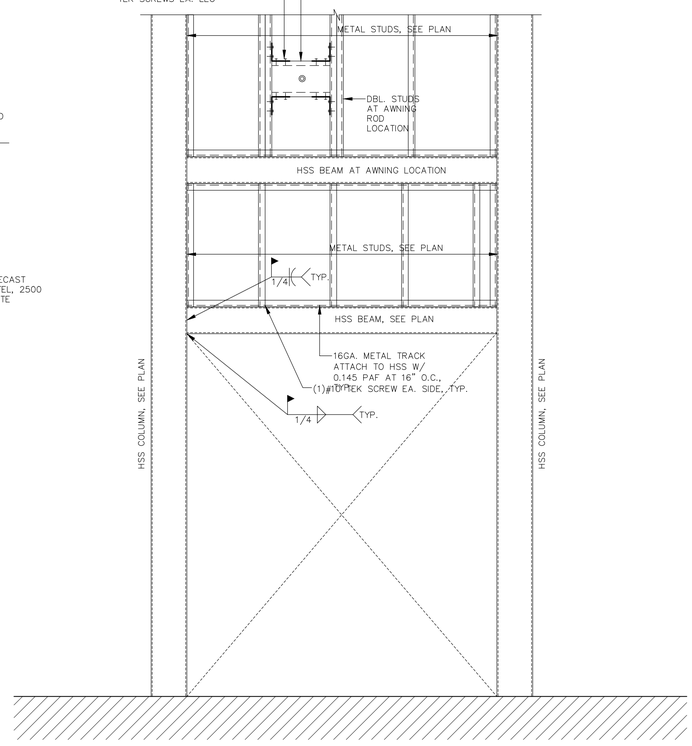


12" C.M.U. PRECAST LINTEL SCHEDULE

N.T.S.

NOTES:

1. SHORE PRECAST LINTEL PER MANUFACTURER RECOMMENDATIONS.
2. VERIFY W/ ARCH'L DWGS. FOR SIZE & LOCATIONS OF OPENINGS.
3. PROVIDE 8" MIN. BEARING @ EACH END.



TYP. STOREFRONT FRAMING ELEVATION

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AR99238
CA 8660

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LIC PEF008252
PROJECT # IP26-38

NO DATE REMARKS

REVISIONS

PHASE I & II – BUILDING DEVELOPMENT

TIMBER DEVELOPMENT CORPORATION

8113 BALL GROUND HWY
BALL GROUND, GA 30107

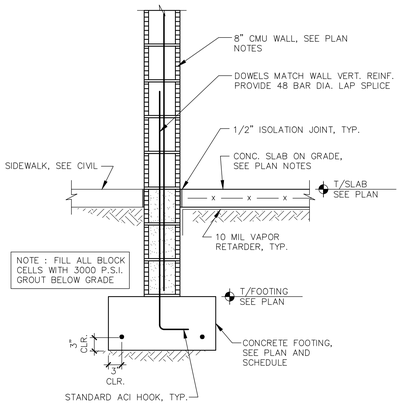
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DATE: 11.02.26

S3.2

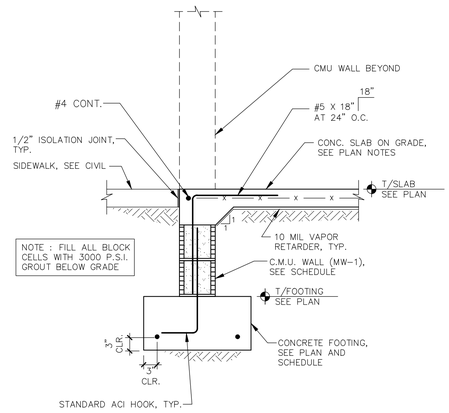
STRUCTURAL DETAILS

CHECKED: BM DRAWN: RED

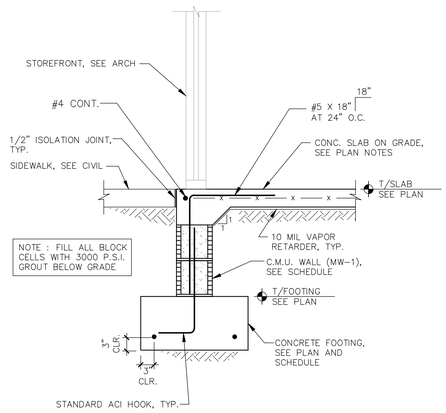
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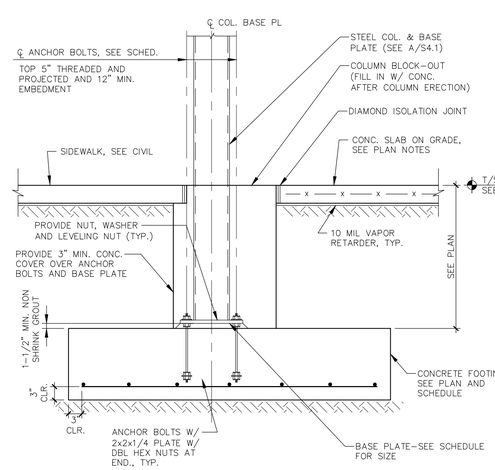
SECTION 1
3/4"=1'-0" S4.1



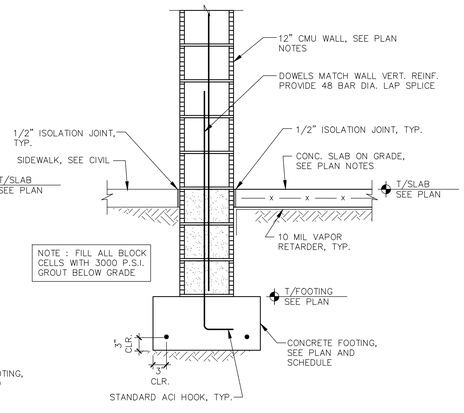
SECTION 2
3/4"=1'-0" S4.1



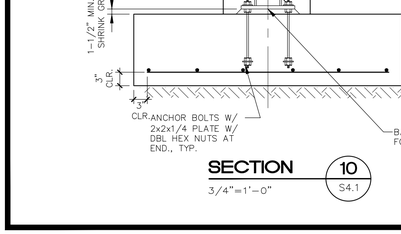
SECTION 3
3/4"=1'-0" S4.1



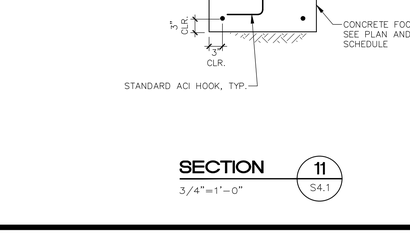
SECTION 4
3/4"=1'-0" S4.1



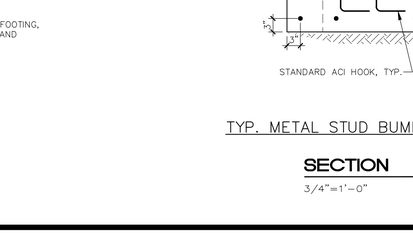
SECTION 5
3/4"=1'-0" S4.1



SECTION 6
3/4"=1'-0" S4.1

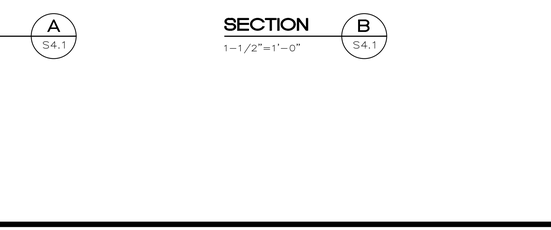


SECTION 7
3/4"=1'-0" S4.1



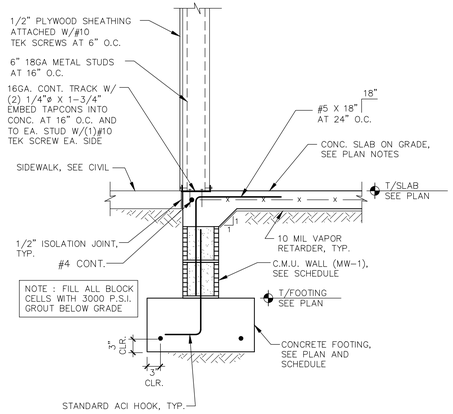
TYP. LOADING DOCK DETAIL

SECTION 8
3/4"=1'-0" S4.1

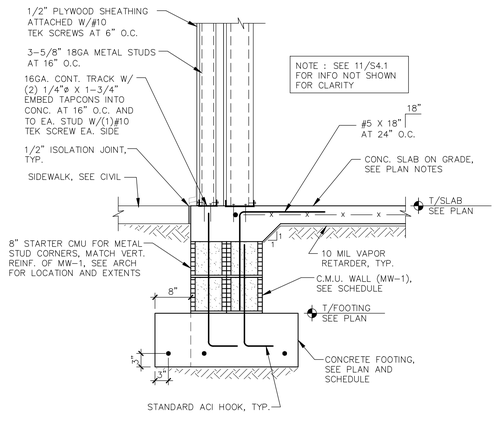


SECTION 9
3/4"=1'-0" S4.1

SECTION 10
3/4"=1'-0" S4.1

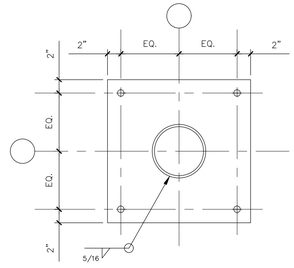


SECTION 11
3/4"=1'-0" S4.1



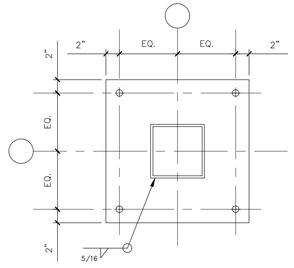
TYP. METAL STUD BUMPOUT DETAIL

SECTION 12
3/4"=1'-0" S4.1



BASE PLATE DETAIL

SECTION A
1-1/2"=1'-0" S4.1



BASE PLATE DETAIL

SECTION B
1-1/2"=1'-0" S4.1

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PROJECT # IP26-38

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PHASE I & II – BUILDING DEVELOPMENT

TIMBER DEVELOPMENT CORPORATION

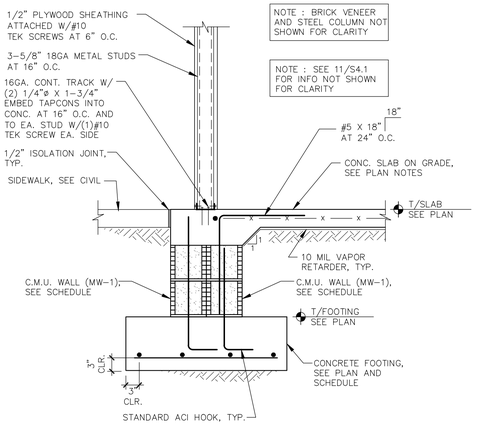
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PROJECT NO: 2026.0771
DATE: 11.02.26

S4.1
FOUNDATION SECTIONS

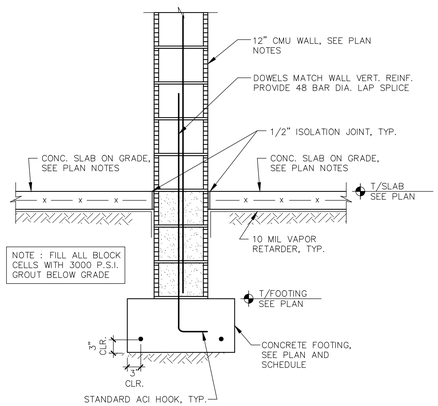
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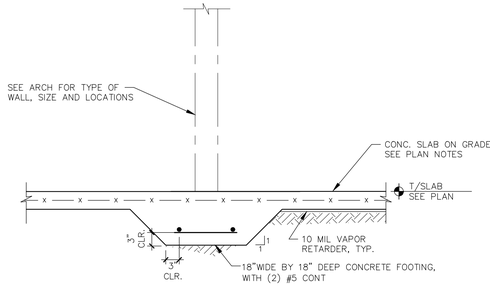


TYP. METAL STUD BOX COLUMN

SECTION 1 — 3/4"=1'-0" — S4.2

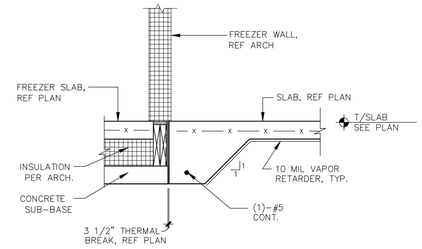


SECTION 2 — 3/4"=1'-0" — S4.2



TYP. INTERIOR PARTITION WALL FTG (TS-1).

SECTION 3 — 3/4"=1'-0" — S4.2



TYP. FREEZER JOINT SECTION

SECTION 4 — 3/4"=1'-0" — S4.2

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PHASE I & II – BUILDING DEVELOPMENT

TIMBER DEVELOPMENT CORPORATION

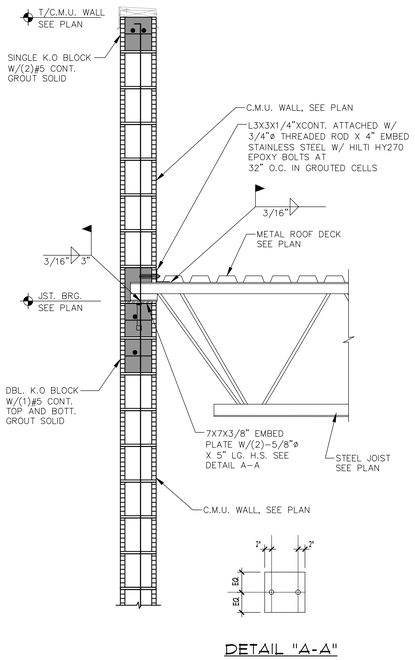
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PROJECT NO: 2026.0771
DATE: 11.02.26

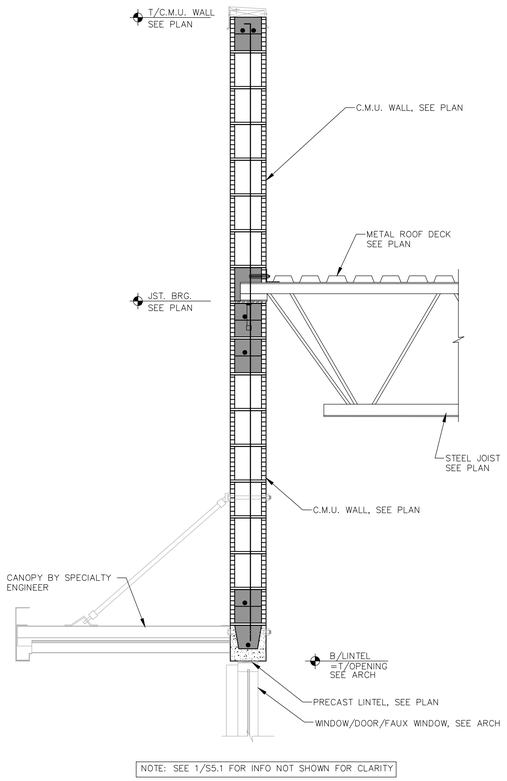
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FOUNDATION SECTIONS

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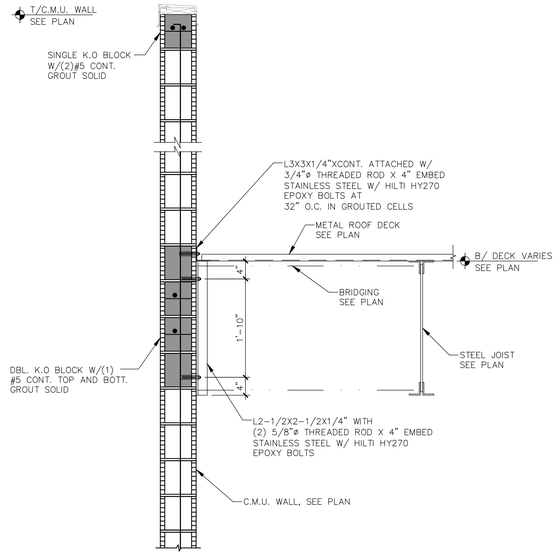
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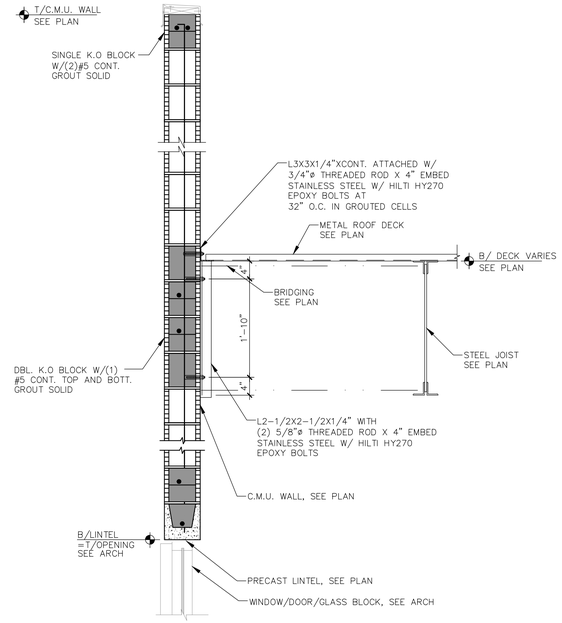


DETAIL "A-A"



NOTE: SEE 1/S5.1 FOR INFO NOT SHOWN FOR CLARITY



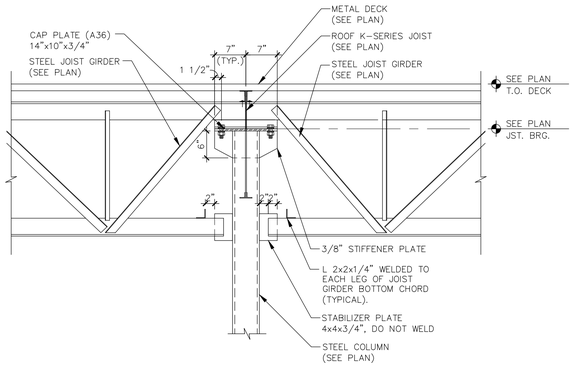


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3/4"=1'-0" S5.1

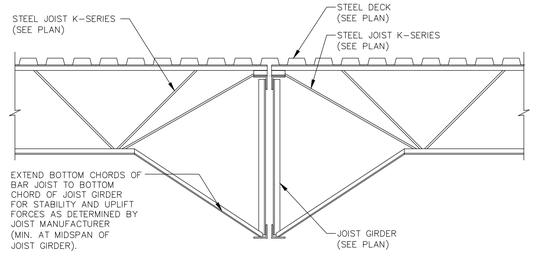
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3/4"=1'-0" S5.1

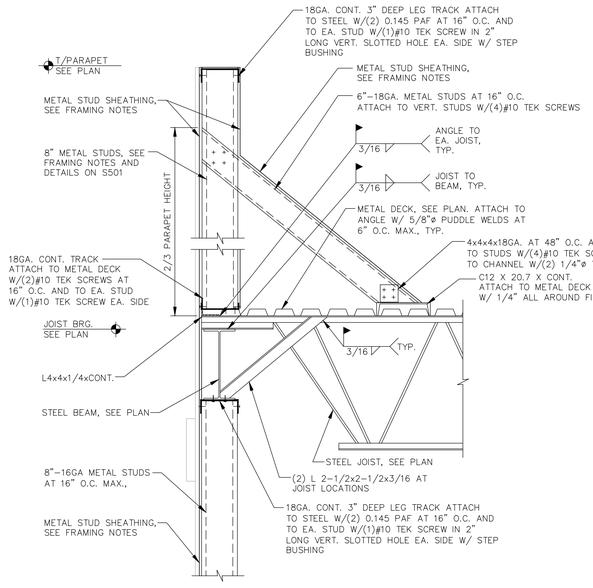
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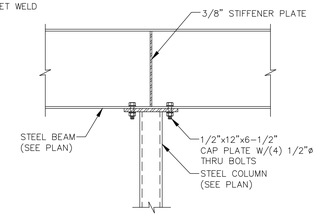
SECTION 5
3/4"=1'-0" S5.1



SECTION 6
3/4"=1'-0" S5.1



SECTION 7
3/4"=1'-0" S5.1



SECTION 8
3/4"=1'-0" S5.1

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PROJECT # IP26-38

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PHASE I & II – BUILDING DEVELOPMENT

TIMBER DEVELOPMENT CORPORATION

8113 BALL GROUND HWY
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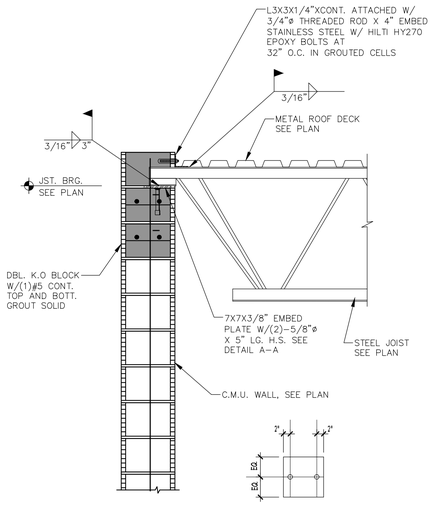
PROJECT NO: 2026.0771
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S5.1

FRAMING SECTIONS

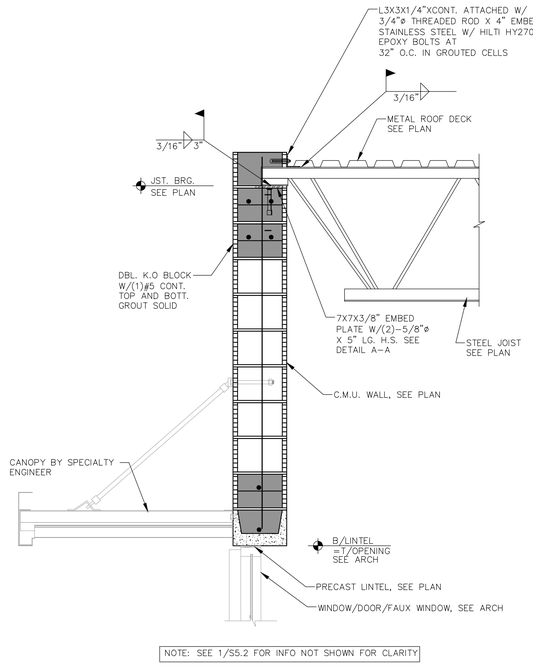
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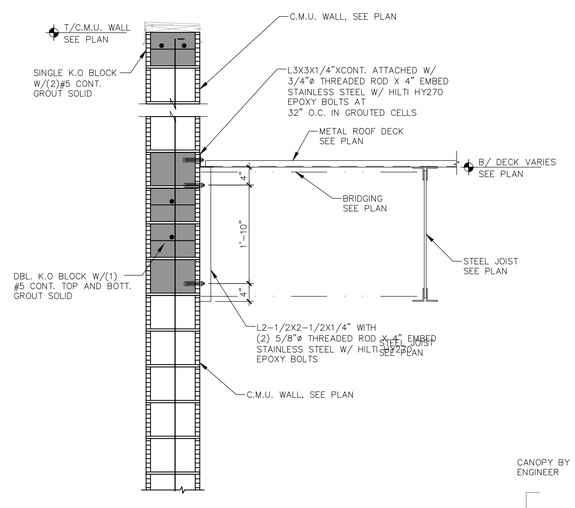
DETAIL "A-A"

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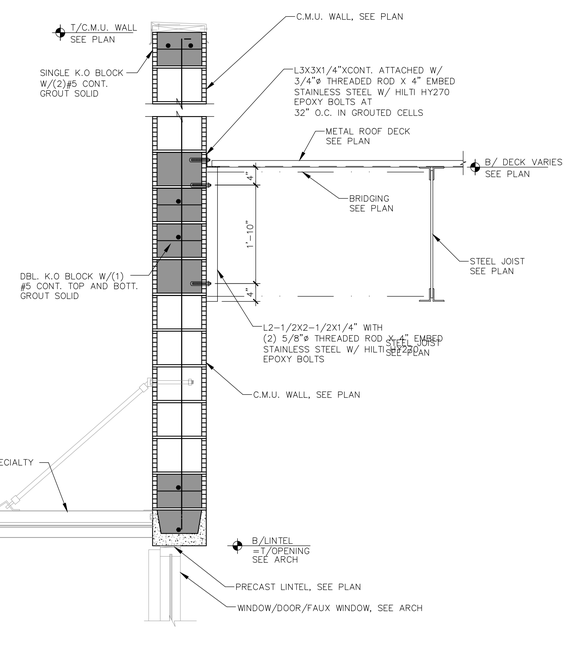


NOTE: SEE 1/S5.2 FOR INFO NOT SHOWN FOR CLARITY

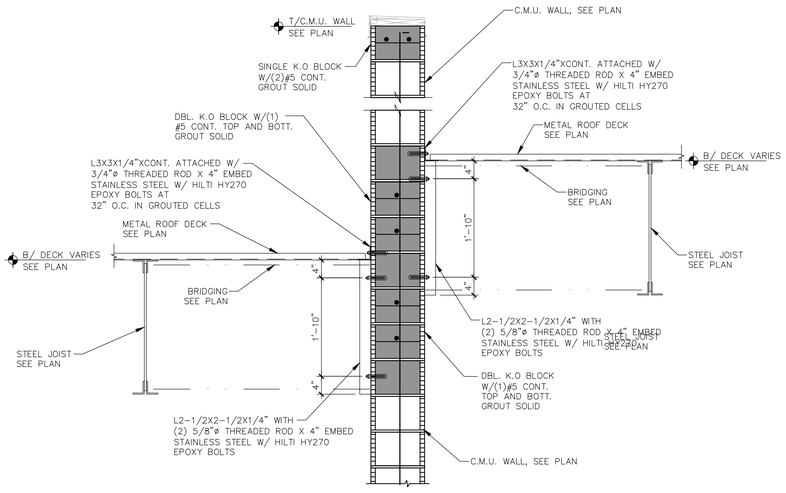
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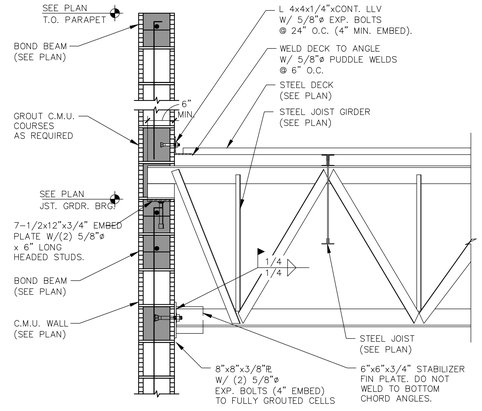
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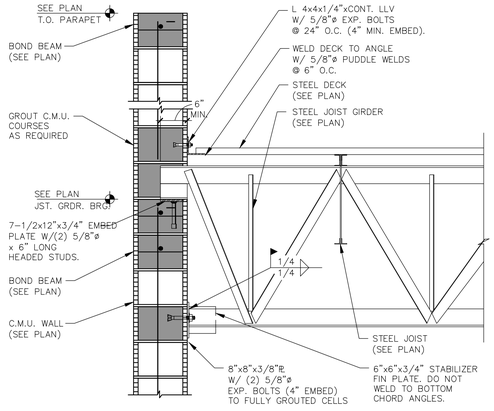
SECTION 4
3/4"=1'-0" — S5.1



SECTION 5
3/4"=1'-0" — S5.1



SECTION 6
3/4"=1'-0" — S5.1



SECTION 7
3/4"=1'-0" — S5.1

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PROJECT # IP26-38

NO DATE REMARKS
REVISIONS

PHASE I & II – BUILDING DEVELOPMENT

TIMBER DEVELOPMENT CORPORATION

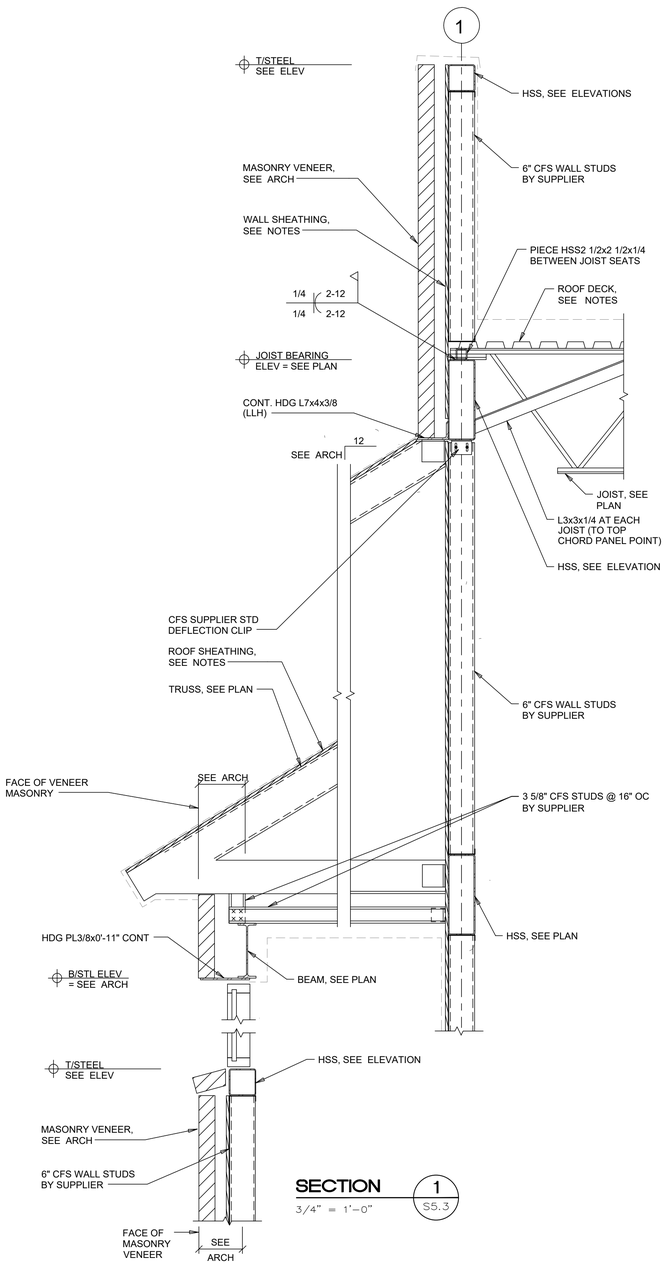
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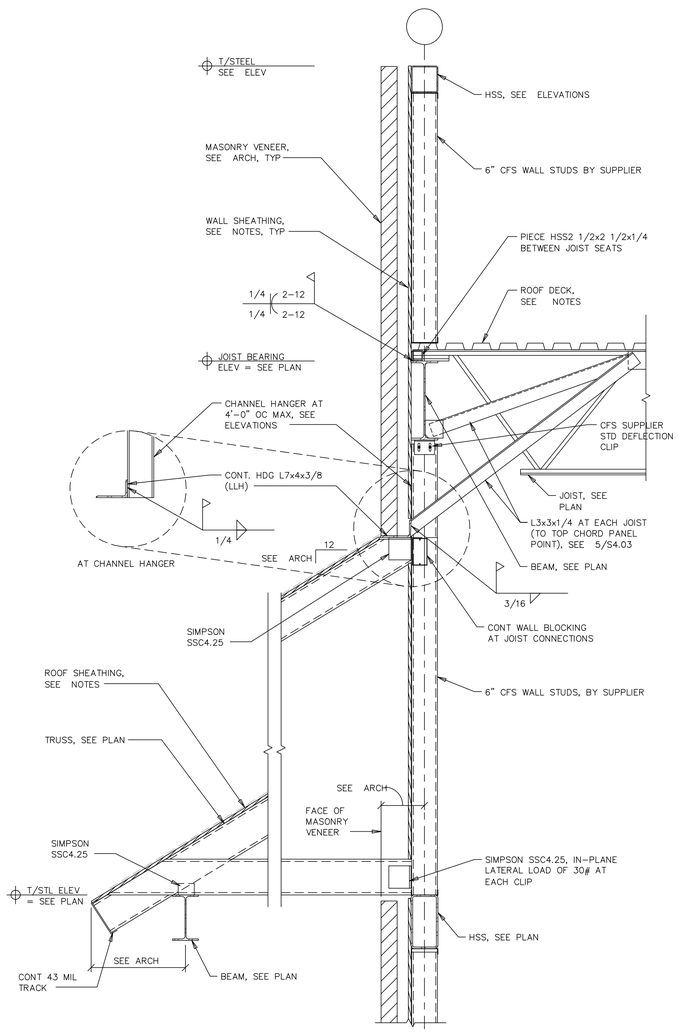
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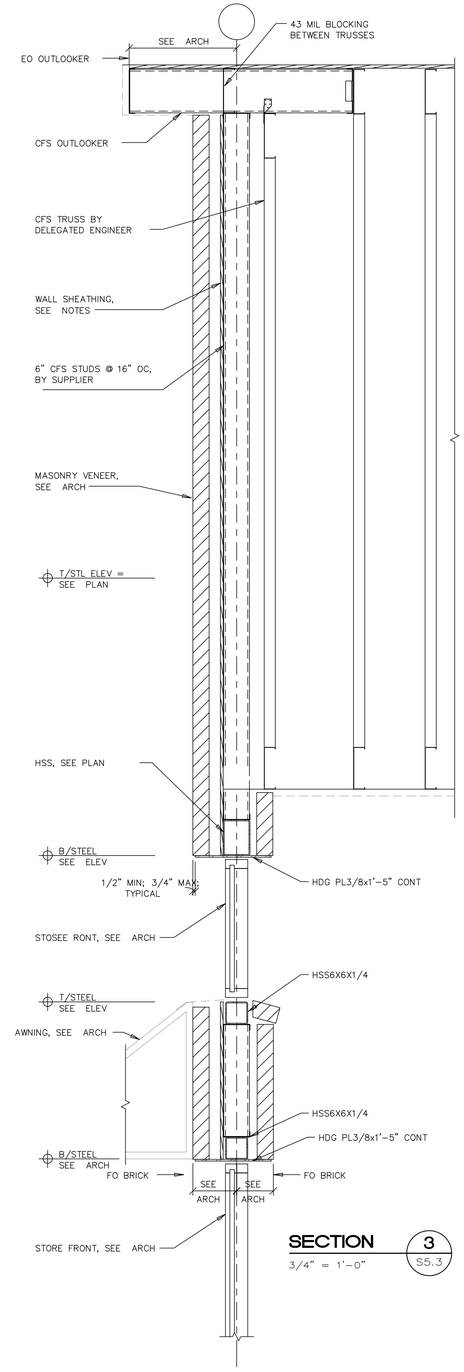
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SECTION 1 S5.3
3/4" = 1'-0"



SECTION 2 S5.3
3/4" = 1'-0"



SECTION 3 S5.3
3/4" = 1'-0"

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PHASE I & II –
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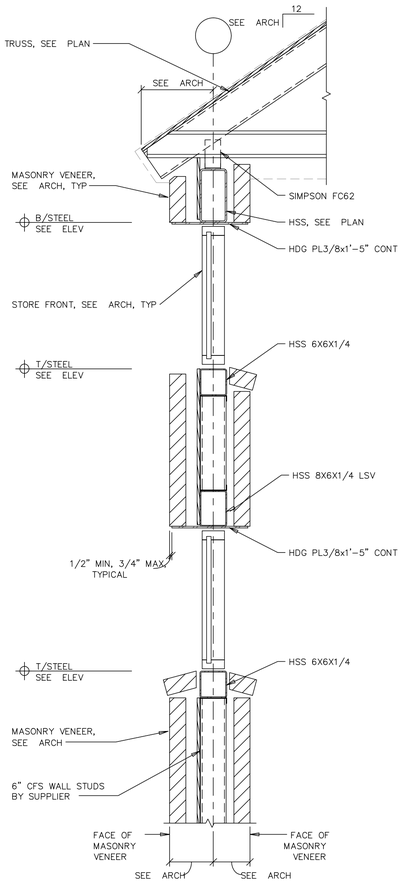
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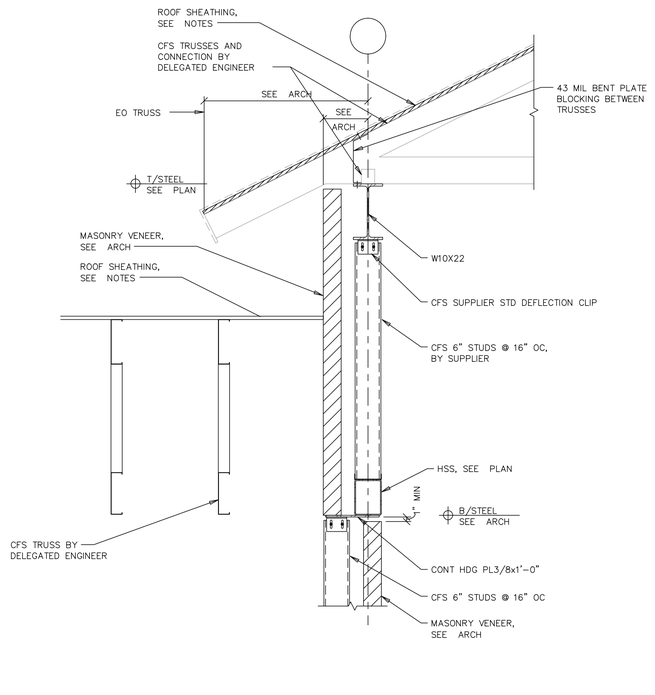
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SECTION 2 / S5.4
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