ABBREVIATIONS

AB	ANCHOR BOLT
ACI	AMERICAN CONCRETE INSTITUTE
AFF	ABOVE FINISHED FLOOR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
ARCH	ARCHITECTURAL
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
BFF	BELOW FINISHED FLOOR
BFG	BELOW FINISHED GRADE
BL	BLOCK LINTEL
BLDG	BUILDING
BM	BEAM
BN	BOUNDARY NAILING
BO	BOTTOM OF
BOD	BOTTOM OF DECK
BOF	BOTTOM OF FOOTING
BOM	BOTTOM OF MASONRY
BOS	BOTTOM OF STEEL
BOT	BOTTOM
BRG	BEARING
CEN	CONTINUOUS EDGE NAILING
CFMF	COLD FORMED METAL FRAMING
CJ	CONTROL JOINT
CL	CENTERLINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
COND	CONDENSER UNIT
CONST	CONSTRUCTION
CONT	CONTINUOUS
DBL	DOUBLE
DEN	DISCONTINUOUS EDGE NAILING
DIA	DIAMETER
EA	EACH
EDC	ELECTRICAL DISTRIBUTION CENTER
EF	EXHAUST FAN
EIFS	EXTERIOR INSULATION AND FINISH SYSTEM
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
EOR	ENGINEER OF RECORD
EQ	EQUAL
ETR	EXISTING TO REMAIN
EW	EACH WAY
FDN	FOUNDATION
FF	FINISHED FLOOR
FN	FIELD NAILING
FOA	FACE OF ANGLE
FS	FAR SIDE
FTG	FOOTING
FV	FIELD VERIFY
GA	GAGE
GC	GENERAL CONTRACTOR
GYP	GYPSUM BOARD
H	HEIGHT
HDG	HOT DIPPED GALVANIZED
HORIZ	HORIZONTAL
HSA	HEADED STUD ANCHOR
HSS	HOLLOW STRUCTURAL SHAPE
INFO	INFORMATION
ISO	ISOLATION
JBE	JOIST BEARING ELEVATION
JST	JOIST
JT	JOINT
KSI	KIPS PER SQUARE INCH
L	LENGTH
LB	POUNDS
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
MAX	MAXIMUM
MECH	MECHANICAL
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
MTL	METAL
NBPS	NOT BY PRECAST SUPPLIER
NIC	NOT IN CONTRACT
NO	NUMBER
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OH	OPPOSITE HAND
OPP	OPPOSITE
OSL	OUTSTANDING LEG
PAF	POWER ACTUATED FASTENER
PC	PRECAST
PCF	POUNDS PER CUBIC FOOT
PEMB	PRE-ENGINEERED METAL BUILDING
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PLUMB	PLUMBING
PMEJ	PREMOLDED EXPANSION JOINT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
QTY	QUANTITY
RACK	REFRIGERATION RACK
REF	REFER TO
REINF	REINFORCING
REQD	REQUIRED
REV	REVERSE
RO	ROUGH OPENING
RTU	ROOF TOP UNIT
SCHED	SCHEDULE
SDI	STEEL DECK INSTITUTE
SIM	SIMILAR
SJI	STEEL JOIST INSTITUTE
SP	SPACES
SPECS	SPECIFICATIONS
SQ	SQUARE
STRUC	STRUCTURAL
T&B	TOP AND BOTTOM
THK	THICKNESS
TO	TOP OF
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOGB	TOP OF GRADE BEAM
TOM	TOP OF MASONRY
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRANS	TRANSVERSE
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W	WIDTH
WP	WORK POINT

DESIGN DATA

1. BUILDING CODE
   A. BUILDING CODE — 2021 IBC

2. GRAVITY LOADS
   A. DEAD LOADS
      - ROOF — 10 PSF
      - GROUND FLOOR — 55 PSF
      - MEZZANINE — 50 PSF

   B. LIVE LOADS
      1. ROOF — 20 PSF
      2. GROUND FLOOR — 200 PSF
      3. MEZZANINE — 80 PSF

   C. ROOF SNOW LOADS
      1. IMPORTANCE FACTOR (I) — 1.0
      2. SNOW EXPOSURE FACTOR (Ce) — 1.0
      3. ROOF THERMAL FACTOR (Ct) — 1.0
      4. GROUND SNOW LOAD (Pg) — 5.0 PSF
      5. FLAT ROOF SNOW LOAD (Pf) — 3.5 PSF

3. LATERAL LOADS
   A. WIND LOADS
      1. BASIC WIND SPEED (3-SECOND GUST)
         - ULTIMATE WIND SPEED — 107 MPH
         - SERVICE WIND SPEED — 82.4 MPH
      2. RISK CATEGORY — II
      3. WIND EXPOSURE CATEGORY — C
      4. VELOCITY PRESSURE EXPOSURE COEFFICIENT (Kh) — 0.92
      5. INTERNAL PRESSURE COEFFICIENT (GCpi) — (±)0.18
      6. WIDTH OF EDGE ZONE — 17.6 FT

   B. DESIGN WIND PRESURES (SERVICE)
      (LOADS ALREADY 0.6 LOAD COMBINATION FACTOR IN ASCE 7-16, SECTION 2.4.1)
      1. EXTERIOR WALLS (BASED ON 100 SQ FT)
         - INTERIOR ZONE — 13.9 PSF
         - CORNER ZONE — 15.4 PSF
         - PARAPETS — 40.4 PSF
      2. ROOF UPLIFT (NET)
         - POSITIVE ALL ZONES — +12.0 PSF
         - NEGATIVE CORNER ZONE — -31.9 PSF
         - NEGATIVE INTERIOR ZONE — -20.2 PSF
         - NEGATIVE EDGE ZONE — -26.8 PSF

   C. SEISMIC LOADS (SERVICE)
      1. MAPPED SPECTRAL RESPONSE ACCELERATION (Ss) — 0.176%
      2. MAPPED SPECTRAL RESPONSE ACCELERATION (S1) — 0.084%
      3. 5% DAMPED SPECTRAL RESPONSE COEFF. (SDS) — 0.188
      4. 1-SEC PERIOD SPECTRAL RESPONSE COEFF. (SD1) — 0.134
      5. IMPORTANCE FACTOR (I) — 1.0
      6. RISK CATEGORY — II
      7. SITE CLASS — D
      8. SEISMIC DESIGN CATEGORY — C
      9. BASIC STRUCTURAL SYSTEM AND SEISMIC RESPONSE — PER PEMB DESIGNER
      10. RESPONSE MODIFICATION FACTOR — PER PEMB DESIGNER
      11. OVERSTRENGTH FACTOR (Ωo) — PER PEMB DESIGNER
      12. ANALYSIS PROCEDURE — PER PEMB DESIGNER
      13. DESIGN BASE SHEAR, V — PER PEMB DESIGNER

GENERAL:

1. ROOF LIVE LOADS HAVE BEEN REDUCED BASED ON TRIBUTARY AREAS IN ACCORDANCE WITH CODE PROVISIONS.

2. DEAD LOADS HAVE BEEN CALCULATED TO INCLUDE THE ACTUAL WEIGHT OF ALL WORK SHOWN ON THE STRUCTURAL, MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS. NO OTHER EQUIPMENT SHALL BE PLACED ON OR HUNG FROM THE ROOF SYSTEM WITHOUT PRIOR WRITTEN APPROVAL OF THE ENGINEER. ROOF-MOUNTED HVAC UNITS SHALL BE PLACED WITHIN THE DESIGNATED AREAS SHOWN ON THE FRAMING PLANS.

3. CONTRACTOR DOCUMENTS:
   THE GENERAL CONTRACTOR SHALL OBTAIN ALL CONTRACT DOCUMENTS AND LATEST ADDENDA AND SUBMIT SUCH DOCUMENTS TO ALL SUBCONTRACTORS AND MATERIAL SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS, FABRICATION OF ANY STRUCTURAL MEMBERS, AND ERECTION IN THE FIELD.

4. COMPLETE SHOP DRAWINGS FOR THE STRUCTURAL WORK SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW PRIOR TO COMMENCEMENT OF CONSTRUCTION, IN ACCORDANCE WITH THE SPECIFICATIONS. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS PRIOR TO SUBMITTING. STRUCTURAL ENGINEER SHALL HAVE TEN FULL BUSINESS WORKING DAYS FOR REVIEWS. SUCH REVIEW BY THE ARCHITECT/ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR CORRECT FABRICATION AND CONSTRUCTION OF THE WORK.

5. ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREON.

6. ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO THE STRUCTURE OR ANY PART OF THE STRUCTURE DETAILED ON THESE DRAWINGS SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT FOR REVIEW. SHOP DRAWINGS THAT ARE SUBMITTED FOR REVIEW DO NOT CONSTITUTE "IN-WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE BEING SUGGESTED.

7. THE CONTRACTOR SHALL REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS NOT SHOWN AND FOR EXACT LOCATIONS OF ALL ARCHITECTURAL DETAILS. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL SECTIONS WITH THE ARCHITECTURAL SECTIONS AND REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO COMPLETION OF THE SHOP DRAWINGS.

8. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.

9. PRINCIPAL OPENINGS ARE SHOWN ON THE STRUCTURAL DRAWINGS. CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, CURBS, INSERTS AND OTHER OPENINGS NOT SHOWN. THE CONTRACTOR SHALL PROVIDE FOR ALL OPENINGS, WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT. SIZE AND LOCATION OF ALL OPENINGS SHALL BE VERIFIED BY THE CONTRACTOR. ANY DEVIATION FROM OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ARCHITECT'S ATTENTION FOR APPROVAL PRIOR TO CONSTRUCTION.

10. THE STRUCTURAL DRAWINGS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.

11. RESPONSIBILITY OF THE CONTRACTOR FOR STABILITY OF THE STRUCTURE DURING CONSTRUCTION:
    ALL STRUCTURAL ELEMENTS OF THE PROJECT HAVE BEEN DESIGNED BY THE STRUCTURAL ENGINEER TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES THAT COULD OCCUR IN THE FINAL COMPLETED STRUCTURE ONLY. THE CONTRACTOR SHALL PROVIDE ALL REQUIRED BRACING AND SHORING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE STABILITY-PROVIDING SYSTEM IS COMPLETELY INSTALLED AND THE STRUCTURE IS COMPLETELY TIED TOGETHER.

12. CONFLICTS IN STRUCTURAL REQUIREMENTS:
    WHERE CONFLICT EXISTS AMONG THE VARIOUS PARTS OF THE STRUCTURAL CONTRACT DOCUMENTS, STRUCTURAL DRAWINGS, GENERAL NOTES, AND SPECIFICATIONS, THE STRICTEST REQUIREMENTS, AS INDICATED BY THE ENGINEER, SHALL GOVERN.

13. STABILITY AND BRACING OF CONCRETE TILT OR MASONRY WALLS DURING CONSTRUCTION:
    THE CONTRACTOR SHALL ADEQUATELY BRACE AND SHORE ALL CONCRETE TILT OR MASONRY WALLS AT ALL STAGES OF CONSTRUCTION TO RESIST ERECTION LOADS AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF CONSTRUCTION.

14. CONTRACTOR SUBSTITUTIONS:
    MATERIALS OR PRODUCTS SUBMITTED FOR APPROVAL THAT DIFFER FROM THE MATERIAL OR PRODUCTS SPECIFIED IN THE STRUCTURAL CONTRACT DOCUMENTS WILL BE APPROVED ONLY IF THE FOLLOWING CRITERIA ARE SATISFIED:
    A. A COST SAVINGS TO THE OWNER IS DOCUMENTED AND SUBMITTED WITH REQUEST.
    B. THE MATERIAL OR PRODUCT COMPLIES WITH THE SAME ASTM SPECIFICATIONS.

15. SITE OBSERVATION BY THE STRUCTURAL ENGINEER:
    THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND, EXCEPT WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES AND SEQUENCE. THE ENGINEER SHALL NOT HAVE CONTROL NOR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSION OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF JFTE, L.P. IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR PROCEEDS IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.

16. CONSTRUCTION MATERIALS SHALL NOT BE STORED ON FLOORS OR ROOF IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENFORCE THIS REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.

17. MAINTENANCE STATEMENT:
    PERIODIC MAINTENANCE IS REQUIRED TO ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL INCLUDE, BUT IS NOT LIMITED TO, SUCH ITEMS AS, PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATING FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE.

EARTHWORK AND FOUNDATIONS:

1. THE FOUNDATION DESIGN IS BASED ON A SUBSURFACE EXPLORATION AND REPORT BY GEO-HYDRO ENGINEERS DATED JANUARY 27, 2023 REPORT No. 222737.20.

2. THE EARTHWORK AND FOUNDATION NOTES CONTAINED HERE ARE INTENDED TO PROVIDE A SUMMARY OF THE RECOMMENDATIONS MADE IN THE GEOTECHNICAL REPORT. CONTRACTOR SHALL READ AND BE THOROUGHLY FAMILIAR WITH ALL RECOMMENDATIONS OF THE REPORT. OMISSIONS OR RESTATEMENTS DO NOT RELIEVE THE CONTRACTOR FROM FULL COMPLIANCE WITH THE REPORT.

3. FOUNDATIONS SHALL BE SHALLOW SPREAD FOOTINGS FOUNDED APPROXIMATELY 18" BELOW FINAL GRADE. ALLOWABLE END BEARING PRESSURE IS 2.5 KSF TOTAL LOAD.

4. EXTERIOR FOOTINGS SHALL BEAR AT OR BELOW MINIMUM BEARING DEPTH. MINIMUM BEARING DEPTH IS 24 INCHES BELOW ADJACENT FINISHED GRADE. THICKENED SLAB EDGES FOR STOOPS, CANOPIES, ETC. SHALL BE 16 INCHES (UNO).

5. STANDARD PROCEDURES OF FROST PROTECTION FOR FOUNDATIONS AND EXCAVATIONS SHALL BE EMPLOYED FOR WINTER CONSTRUCTION. BACKFILLING OF EXCAVATIONS SHALL BE DONE AS SOON AS POSSIBLE TO PROTECT FOUNDATIONS FROM FROST.

6. CONCRETING OF EACH DRILLED SHAFT SHALL BE COMPLETED WITHIN 5 HOURS OF EXCAVATION. IF GROUNDWATER SEEPAGE IS ENCOUNTERED DURING PIER CONSTRUCTION, CASING MAY BE REQUIRED. ANY EXCESS WATER SHALL BE REMOVED PRIOR TO PLACING CONCRETE.

7. EXCAVATION FOR THE GRADE BEAMS/ STEM WALLS AT THE EXTERIOR WALLS SHALL BE BACK FILLED PER THE GEOTECH REPORT.

8. EXISTING SUBGRADE UNDER SLAB AREAS SHALL BE REMOVED AND REPLACED PER THE GEOTECHNICAL ENGINEERS RECOMMENDATIONS GIVEN IN THE REPORT NOTED ABOVE TO ACHIEVE A MAXIMUM THEORETICAL PVR OF 1 INCH.

CAST IN PLACE REINFORCED CONCRETE:

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT EDITION OF A.C.I. 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".

2. MIXES SHALL BE DESIGNED TO PROVIDE CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH AS NOTED BELOW (NO CALCIUM CHLORIDE SHALL BE PERMITTED).
   (a) UNLESS NOTED OTHERWISE, ALL CONCRETE — f'c = 3,000 P.S.I.
   (b) CONCRETE SLAB-ON-GRADE — f'c = 4,000 P.S.I.
   (c) CONCRETE SLAB ON METAL DECK — f'c = 3,000 P.S.I. LIGHTWEIGHT
   (d) TILT WALL PANELS — f'c = 4,000 P.S.I.

3. PORTLAND CEMENT SHALL BE A SINGLE BRAND CONFORMING TO ASTM C-150, E I/II.

4. NORMAL WEIGHT (150 PCF CONCRETE) AGGREGATES SHALL CONFORM TO ASTM C-33 AND SHALL BE FROM A SINGLE SOURCE FOR EXPOSED CONCRETE. ALL CONCRETE SHALL UTILIZE NORMAL WEIGHT AGGREGATE UNLESS NOTED OTHERWISE.

5. ALL ADDITIVES FOR AIR ENTRAINMENT, WATER REDUCTION, AND SET CONTROL SHALL BE USED IN ACCORDANCE WITH THE MANUFACTURER'S DIRECTIONS, PROJECT SPECIFICATIONS, AND WEATHER CONDITIONS.

6. THE MAXIMUM NOMINAL SIZES OF COURSE AGGREGATE BE AS FOLLOWS:
   PIERS/FOOTINGS......................................1 1/2"
   SLABS..3/4"
   ALL OTHER CONCRETE.....................................1"

7. CONCRETE SLUMPS SHALL BE FOUR (4) INCH TO SIX (6) INCH MAXIMUM.

8. CONCRETE SHALL BE CONSOLIDATED WITH A MECHANICAL VIBRATOR WITH A MINIMUM 3/4" DIAMETER HEAD. "RODDING" IS NOT ALLOWED.

9. ALL EXTERIOR CONCRETE SHALL BE AIR ENTRAINED. AIR CONTENT SHALL BE 4 1/2% -/+ 1 1/2%. INTERIOR SLABS SHALL NOT HAVE AIR-ENTRAINMENT.

10. MILD STEEL REINFORCING BARS SHALL CONFORM TO ASTM A-615, WELDABLE REINFORCING SHALL CONFORM TO ASTM A-706. No. 3 BARS SHALL BE GRADE 40. No. 4 BARS AND LARGER SHALL BE GRADE 60. WELDING OF REINFORCING BARS SHALL CONFORM TO THE ANSI/AWS D1.4 WELDING CODE.

11. WELDED STEEL WIRE FABRIC SHALL CONFORM TO ASTM A-185. ALL WELDED STEEL WIRE FABRIC SHALL BE SUPPLIED IN FLAT SHEETS, NOT IN ROLLS.

12. MILD STEEL REINFORCEMENT AND ACCESSORIES SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH ACI SP-66.

13. MILD STEEL REINFORCEMENT SHALL BE PLACED AND SECURED IN ACCORDANCE WITH CRSI "RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS".

14. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE AS FOLLOWS:
    GRADE BEAMS/FOOTINGS......................1 1/2" TOP
    2" SIDES
    3" BOTTOM
    SLAB...3/4" TOP & BOTTOM
    PIERS..3" ALL SURFACES

15. THE TESTING LABORATORY SHALL BE NOTIFIED AFTER THE MILD STEEL REINFORCEMENT AND EMBEDS ARE POSITIONED PRIOR TO EACH CONCRETE PLACEMENT. NO CONCRETE SHALL BE PLACED UNTIL THESE ITEMS ARE CHECKED AND APPROVED BY THE TESTING LABORATORY.

16. EACH AREA OF CONCRETE WORK SHALL BE FINISHED AND CURED IN ACCORDANCE WITH THE SPECIFICATIONS. CHAMFERS SHALL BE PROVIDED IN ACCORDANCE WITH THE ARCHITECTURAL DRAWINGS.

17. CURING OF CONCRETE WORK:
    ALL CONCRETE WORK FORMED AND UNFORMED SHALL BE WET CURED FOR A MINIMUM PERIOD OF SEVEN DAYS.

18. WATERSTOPS SHALL BE 4" PVC DUMBBELL OR CENTER-BULB E CONFORMING TO CORPS OF ENGINEERS C.R.D.-C-572. WATERSTOPS SHALL BE EMPLOYED AT ALL JOINTS BELOW FINISHED GRADE EXCEPT FOR INTERIOR JOINTS IN SLABS-ON-GRADE. WATERSTOPS SHALL BE PROPERLY INSTALLED TO PREVENT COLLAPSE DURING CONCRETE PLACEMENT.

19. REINFORCING BARS No. 11 AND SMALLER SHALL BE CONTACT LAP SPLICED AS INDICATED IN BAR LAP SCHEDULE, UNLESS SHOWN OTHERWISE. WELDED WIRE FABRIC SHALL BE SPLICED BY LAPPING ONE FULL MESH AND LACING THE SPLICES WITH WIRE. ALL SPLICES SHALL BE STAGGERED.

LAP SPLICE SCHEDULE

BAR SIZE	LAP	BAR SIZE	LAP
3	1'-10"	7	3'-9"
4	2'-3"	8	4'-9"
5	2'-8"	9	6'-1"
6	3'-2"		

20. PROVIDE 90° STANDARD HOOKS WHERE SHOWN IN DETAILS, MINIMUM LENGTH SHALL BE 12db, WHERE db IS DIAMETER OF BAR.

21. SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE SHALL BE TAKEN NOT LESS THAN THE FOLLOWING:
    a) ONCE PER DAY
    b) ONCE PER EVERY 150 CUBIC YARDS
    c) ONCE PER EVERY 5000 SQUARE FEET OF SURFACE AREA FOR SLABS OR WALLS

22. IF TOTAL VOLUME OF CONCRETE IS SUCH THAT FREQUENCY OF TESTING REQUIRED BY NOTE ABOVE WOULD PROVIDE LESS THAN 5 STRENGTH TESTS FOR EACH CLASS OF CONCRETE, TESTS SHALL BE MADE FROM AT LEAST 5 RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN 5 BATCHES ARE USED.

POST-INSTALLED ANCHORS:

1. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.

   A. EXCEPT WHERE INDICATED ON THE DRAWINGS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHORS AS PROVIDED BY HILTI, INC. CONTACT HILTI AT (800) 879-8000 FOR PRODUCT RELATED QUESTIONS.

   I. ANCHORAGE TO CONCRETE

      A. ADHESIVE ANCHORS FOR CONCRETE USE:
         1. HILTI HIT-RE 500-SD EPOXY ADHESIVE ANCHORING SYSTEM PER ICC ESR-2322 FOR SLOW CURE APPLICATIONS.
         2. HILTI HIT-HY 200 SAFE SET SYSTEM WITH THE HILTI HIT-Z ROD PER ICC ESR-3187 OR TRADITIONAL HOLE CLEANING PER MANUFACTURER.
         3. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) WITH HAS-E THREADED ROD PER ICC ESR-3187.
         4. STEEL ANCHOR ELEMENT SHALL BE HILTI HIS-N INTERNALLY THREADED INSERTS (USED WITH RE 500-SD AND HY 200 MAX ONLY), HILTI HAS-E CONTINUOUSLY THREADED ROD, OR CONTINUOUSLY DEFORMED STEEL REBAR.

      B. MEDIUM DUTY MECHANICAL ANCHORS FOR CONCRETE USE:
         HILTI KWIK HUS EZ AND KWIK HUS EZ-I SCREW ANCHORS PER ICC ESR-3027
         HILTI KWIK BOLT-TZ EXPANSION ANCHORS PER ICC ESR-1917

      C. HEAVY DUTY MECHANICAL ANCHORS FOR CONCRETE USE:
         HILTI HDA UNDERCUT ANCHORS PER ICC ESR 1546
         HILTI HSL-3 EXPANSION ANCHORS PER ICC ESR 1545

   II. REBAR DOWELING INTO CONCRETE

      A. ADHESIVE ANCHORS FOR CONCRETE USE:
         1. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) WITH CONTINUOUSLY DEFORMED REBAR PER ICC ESR-3187. OR USE TRADITIONAL HOLE CLEANING PER MANUFACTURER.

   III. ANCHORAGE TO SOLID GROUTED MASONRY

      A. ADHESIVE ANCHORS USE:
         1. HILTI HIT-HY 200 MAX ADHESIVE ANCHORING SYSTEM PER ICC ESR-3187.
         2. HILTI HIT-HY 70 MASONRY ADHESIVE ANCHORING SYSTEM PER ICC ESR-2682.
         3. STEEL ANCHOR ELEMENT SHALL BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR CONTINUOUSLY DEFORMED STEEL REBAR.

      B. MECHANICAL ANCHORS USE:
         HILTI KWIK BOLT-3 EXPANSION ANCHORS PER ICC ESR 1385

      C. ANCHORAGE TO HOLLOW / MULTI-WYTHE MASONRY

      D. ADHESIVE ANCHORS USE:
         1. HILTI HIT-HY 70 MASONRY ADHESIVE ANCHORING SYSTEM PER ICC ESR-2682.
         2. STEEL ANCHOR ELEMENT SHALL BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR CONTINUOUSLY DEFORMED STEEL REBAR.
         3. THE APPROPRIATE SIZE SCREEN TUBE SHALL BE USED PER ADHESIVE MANUFACTURER'S RECOMMENDATION.

2. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.

3. INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.

4. OVERHEAD ADHESIVE ANCHORS MUST BE INSTALLED USING THE HILTI PROFI SYSTEM.

5. THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.

6. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.

7. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY HILTI FERROSCAN, GPR, X-RAY, CHIPPING OR OTHER MEANS.

PREFABRICATED METAL BUILDING:

1. ALL ASPECTS OF SUPERSTRUCTURE BY PRE-ENGINEERED METAL BUILDING MANUFACTURER (PEMB) SHALL BE DESIGNED BY AN ENGINEER LICENSED IN THE STATE OF THE PROJECT CONFORMING TO THE BUILDING CODE NOTED UNDER DESIGN DATA.

2. SUBMITTAL DRAWINGS SHALL BEAR SIGNATURE AND SEAL OF ENGINEER, AND SHALL INCLUDE COLUMN REACTIONS FOR STRUCTURAL ENGINEER TO UTILIZE FOR FOUNDATION DESIGN OR THEY WILL BE RETURNED AND REJECTED.

3. ROOF DEAD LOADS — ACTUAL DL OF STRUCTURE/DECK/ROOFING/ETC
   ROOF TOP UNITS PER MECH PLANS/SCHEDULES
   10 PSF COLLATERAL FOR LIGHTS/CEILING/MISC

   ROOF LIVE LOAD — 20 PSF (REDUCIBLE PER IBC CODE LISTED UNDER DESIGN DATA)

   ROOF SNOW LOAD — AS NOTED IN DESIGN DATA (DRIFT AS REQ'D)

4. DEFLECTION AND STORY DRIFT LIMITS

	DL	S OR W	DL+LL
ROOF MEMBERS:			
SUPPORTING PLASTER OR STUCCO CEILING	L/360	L/360	L/240
SUPPORTING NONPLASTER CEILING	L/240	L/240	L/180
NOT SUPPORTING CEILING	L/180	L/180	L/120
FLOOR MEMBERS	L/360	--	L/240
EXTERIOR WALLS/INTERIOR PARTITIONS:			
WITH PLASTER OR STUCCO FINISHES	--	L/360	--
WITH BRITTLE FINISHES	--	L/240	--
WITH FLEXIBLE FINISHES	--	L/120	--
BUILDING STORY DRIFT	--	H/200	--

STATEMENT OF SPECIAL INSPECTIONS

SPECIAL INSPECTIONS ARE REQUIRED. REFER TO THE APPLICABLE PROJECT SPECIFICATION SECTIONS FOR THE FOLLOWING INFORMATION REGARDING THE REQUIREMENTS OF SPECIAL INSPECTIONS:

1. THE MATERIALS, SYSTEMS, COMPONENTS AND WORK REQUIRED TO HAVE SPECIAL INSPECTIONS.
2. THE TYPE AND EXTENT OF EACH SPECIAL INSPECTION.
3. THE TYPE AND EXTENT OF EACH TEST.
4. ADDITIONAL SPECIAL INSPECTION REQUIREMENTS FOR WIND OR SEISMIC RESISTANCE (WHEN APPLICABLE).
5. THE FREQUENCY OF SPECIAL INSPECTIONS AND TESTING.

THE SPECIAL INSPECTION REQUIREMENTS ARE BASED ON CHAPTER 17 OF THE IBC. THE EXEMPTIONS ALLOWED FOR CONCRETE CONSTRUCTION HAVE BEEN UTILIZED WHERE APPLICABLE.

SPECIAL INSPECTIONS - SOILS AND FOUNDATIONS TABLE

ITEM	INSPECTION FREQUENCY	SCOPE
SOILS	PERIODIC	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY; VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL; PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS; PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY
SOILS	CONTINUOUS	VERIFY USE OF PROPER MATERIALS, DENSITIES, LIFT THICKNESSES, AND COMPACTION OF FILL;VERIFY MATERIALS AND PROCEDURES COMPLY WITH THE GEOTECHNICAL REPORT

SPECIAL INSPECTIONS - CONCRETE TABLE

ITEM	INSPECTION FREQUENCY	SCOPE
REINFORCEMENT	PERIODIC	INSPECT REINFORCEMENT (INCLUDING PRESTRESSING TENDONS) AND PLACEMENT; VERIFY CONFORMANCE WITH CONSTRUCTION DOCUMENTS, AND THAT BARS ARE FREE FROM MATERIALS THAT COULD PREVENT BOND, ARE ADEQUATELY LAPPED, SPLICED, TIED, AND SUPPORTED
ANCHOR INSTALLATION	PERIODIC	INSPECT CAST-IN-PLACE ANCHORS AND BOLTS
ANCHOR INSTALLATION	PERIODIC	INSPECT POST-INSTALLED MECHANICAL AND ADHESIVE ANCHORS NOT OTHERWISE SPECIFIED
ANCHOR INSTALLATION	CONTINUOUS	INSPECT POST-INSTALLED MECHANICAL AND ADHESIVE ANCHORS PER THE REQUIREMENTS IN THEIR RESPECTIVE ICC-ES REPORTS
MIX DESIGN	PERIODIC	VERIFY USE OF APPROVED MIX DESIGN
SAMPLING AND TESTING	CONTINUOUS	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTING; PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE TEMPERATURE OF THE CONCRETE
CONCRETE PLACEMENT	PERIODIC	INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF CONCRETE MEMBER BEING FORMED
CONCRETE PLACEMENT	CONTINUOUS	CONCRETE PLACEMENT

CROWN ENTERPRISES LLC

NOT FOR CONSTRUCTION

NO.:	ISSUED FOR:	DATE
	PERMIT	06/27/2025

OKW Architects

OKW
Architects
600 West Jackson Blvd
Chicago, IL 60661
T 312.798.7700
F 312.798.7777
www.okwarchitects.com

CROWN MORELAND AVENUE MAINTENANCE SHOP

2750 MORELAND AVENUE
ATLANTA, GEORGIA

STRUCTURAL GENERAL NOTES

GEORGIA REGISTERED No. 29469 PROFESSIONAL ENGINEER GRANT COU[illegible]
06.27.2025

JFTE
James F. Turner Engineers, L.P.
Consulting Engineers
8340 Meadow Road Suite 160
Dallas, TX 75231
TEL. 214-750-2900
FAX 214-750-2902
FIRM # PEF007329
Job Number: 55015.01

DRAWN HPL | DESIGN CW | QC GC | APPD GC

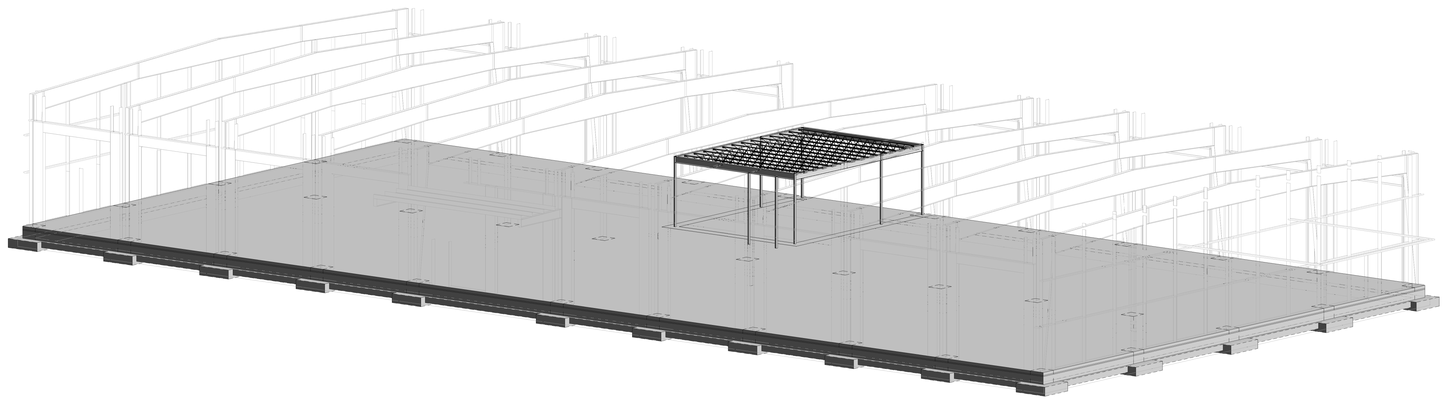
DRAWN BY: HL CHECKED: GC

DATE: 08/13/12

SHEET NO.: S000

PROJ. NO.: Project Number

© 2022 OKW ARCHITECTS



1 FOUNDATION ISOMETRIC VIEW

CROWN ENTERPRISES LLC

NOT FOR CONSTRUCTION

NO.	ISSUED FOR:	DATE
PERMIT		06/27/2025

OKW Architects

OKW
Architects

600 West Jackson Blvd
Chicago, IL 60661
T 312.798.7700
F 312.798.7777
www.okwarchitects.com

CROWN MORELAND AVENUE MAINTENANCE SHOP

2750 MORELAND AVENUE
ATLANTA, GEORGIA

FOUNDATION ISOMETRIC

GEORGIA REGISTERED No. 29489 PROFESSIONAL ENGINEER GRANT COURTNEY

06.27.2025

James F. Turner Engineers, L.P.
Consulting Engineers
5340 Alkahest Road Suite 160
Frisco, TX 75034
TEL 214-750-2900
FAX 214-750-2902
FIRM # REF001309

Job Number: 55015.01

DRAWN	HPL	DESIGN	CW	QC	GC	APPD	GC

DRAWN BY: HL CHECKED: GC

DATE: 05/03/23

SHEET NO.: S100A

PROJ. NO.: **Project Number**

© 2021 OKW ARCHITECTS

PLAN NOTES:

1. A. CIVIL DATUM ELEVATION 876.50' = FINISHED FLOOR ELEVATION. B. ELEVATIONS SHOWN ARE RELATIVE TO DATUM ELEV 100'-0"
2. REFERENCE ARCHITECTURAL DRAWINGS FOR DOOR OPENING DIMENSIONS.
3. PROVIDE POSITIVE TIE BETWEEN FOOTING REINFORCING, STEM WALL REINFORCING, AND BUILDING COLUMN ANCHOR BOLTS FOR GROUNDING CONTINUATION.
4. "CJ" INDICATES SAW CUT CONSTRUCTION JOINT IN SLAB-ON-GRADE. CONSTRUCTION JOINT IN SLAB-ON-GRADE AS REQUIRED BY CONTRACTOR. CONTRACTOR TO LOCATE CONTROL JOINTS AT GRID AND AT 12'-0" OC MAXIMUM SPACING UNLESS NOTED OTHERWISE ON THE CONTRACT DRAWINGS. REF 2/S200. VERIFY LAYOUT WITH OWNER PRIOR TO FIELD WORK BEING COMPLETED.
5. PROVIDE (2) #4 BARS, 2'-0" LONG DIAGONALLY IN TOP OF SLAB AT RE-ENTRANT CORNERS REF 5/S200.
6. REFER TO MECHANICAL DRAWINGS FOR FLOOR DRAIN LOCATIONS, REF 3/S200.
7. PROVIDE ISOLATION JOINT ON EXTERIOR FACE OF STUD. REF 6/S200.
8. REF 7/S200 & 8/S200 FOR PIPE PENETRATION DETAILS.

CONTRACTOR NOTES:

1. FOUNDATION DESIGN IS PRELIMINARY UNTIL FINAL COLUMN REACTIONS ARE REVIEWED BY THE ENGINEER OF RECORD. UPON REVIEW, THE FOUNDATIONS WILL BE ANALYZED AND MODIFIED AS NECESSARY. CONTRACTOR SHALL NOT CONSTRUCT FOUNDATIONS UNTIL FINAL PLANS ARE RELEASED. THE PRESENCE OF THIS NOTE ON THE PLANS INDICATES THAT FINAL COORDINATION OF FOUNDATION SIZES HAVE NOT BEEN PERFORMED.

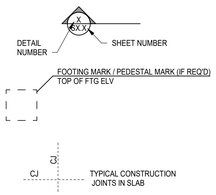
FOOTING SCHEDULE			
FOOTING MARK	SIZE DxWxT	REINFORCING (BOTTOM ONLY)	NOTES
F4	4'-0"x4'-0"x1'-0"	(4) #5 EW T&B	1
F6	6'-0"x6'-0"x1'-6"	(6) #7 EW T&B	1
F7	7'-0"x7'-0"x1'-6"	(7) #7 EW T&B	1
F8.6	8'-0"x6'-0"x1'-6"	(6) #7 LONG T&B (8) #7 TRANS T&B	1
WF2	CONT x2'-0"x1'-4"	(4)#5 CONT W #5 AT 12" OC TRANSVERSE	1 & 2

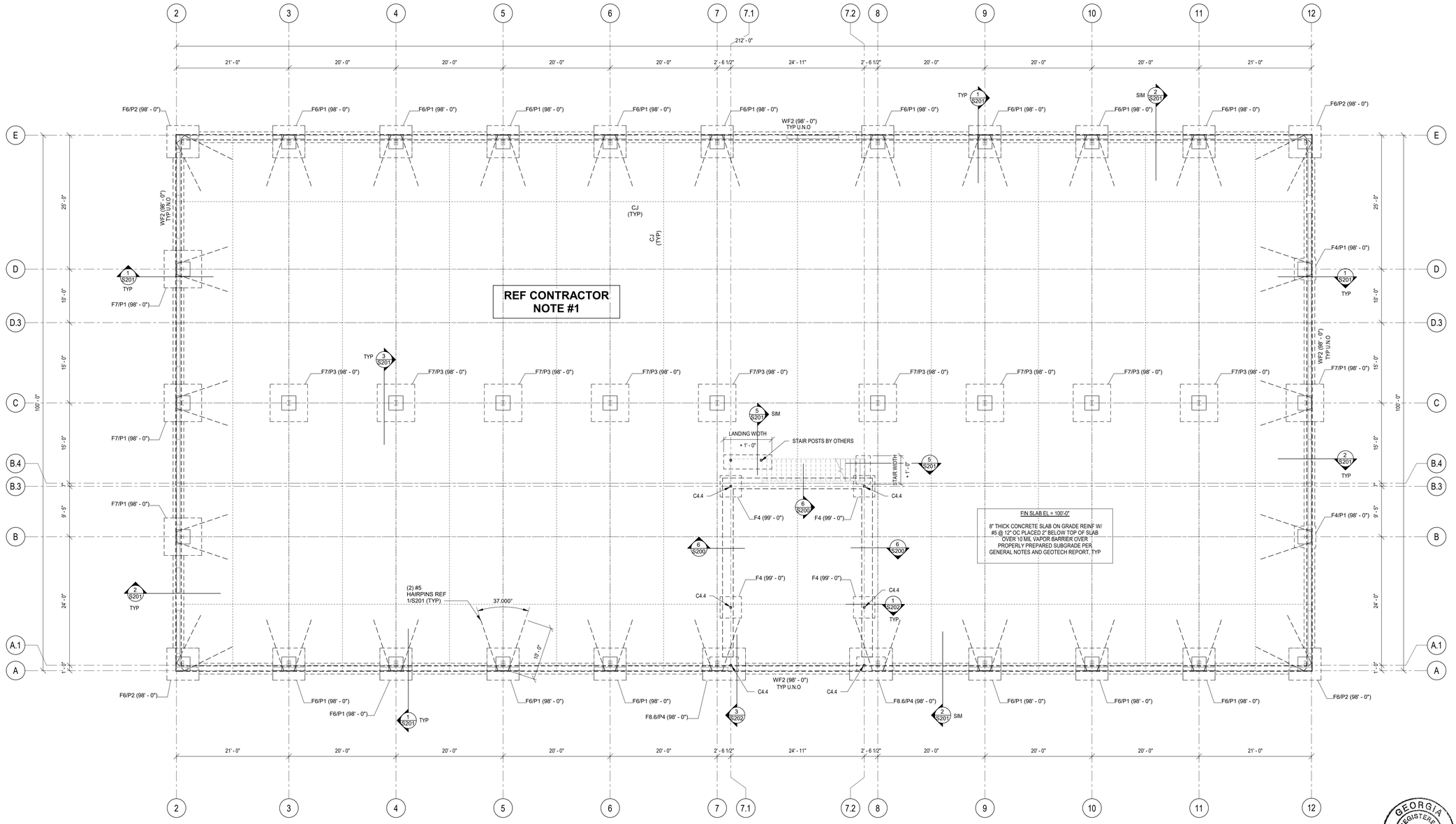
FOOTING NOTES

1. TOP OF FOOTING ELEVATION, REFER PLAN.
2. ALL WALL FOOTINGS WF2 UNO.

COLUMN SCHEDULE	
COLUMN MARK	SIZE
C4.4	HSS4x4x1/4

LEGEND:





1 FOUNDATION PLAN

1/8" = 1'-0"



06.27.2025

James F. Turner Engineers, L.P.
Consulting Engineers
4340 Medow Reed Suite 160
Dallas, TX 7523
PH: 214-750-3900
FAX: 214-750-3902
FIRM # F8253720

Job Number: 55015.01

DRAWN	HPL	DESIGN	CR	QC	GC	APPD	GC

NOT FOR CONSTRUCTION

NO.:	ISSUED FOR:	DATE
PERMIT		06/27/2025

OKW Architects

OKW Architects
600 West Jackson Blvd
Chicago, IL 60661
T 312.798.7700
F 312.798.7777
www.okwarchitects.com

CROWN MORELAND AVENUE MAINTENANCE SHOP

2750 MORELAND AVENUE
ATLANTA, GEORGIA

FOUNDATION PLAN

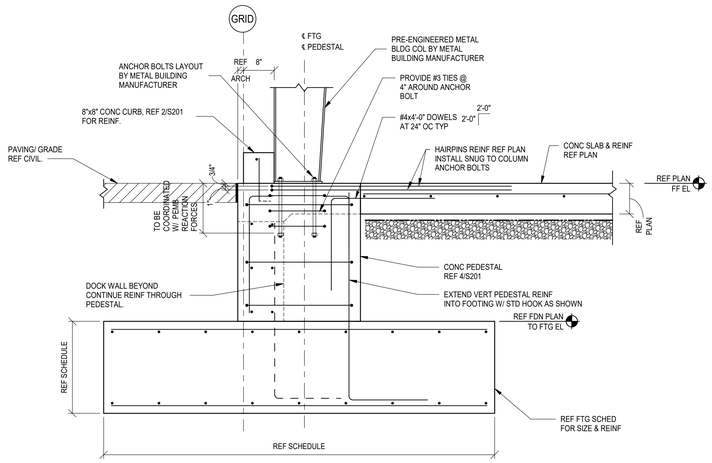
DRAWN BY: HL CHECKED: GC

DATE: 11/13/23

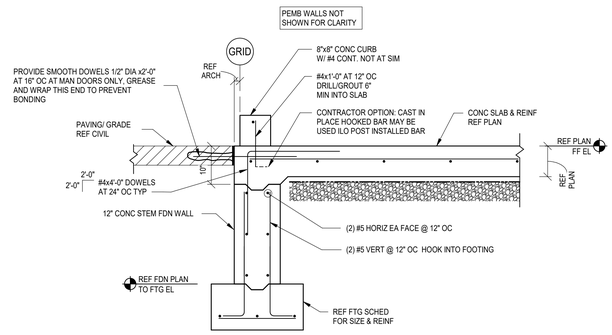
SHEET NO.: S100B

PROJ. NO.: Project Number

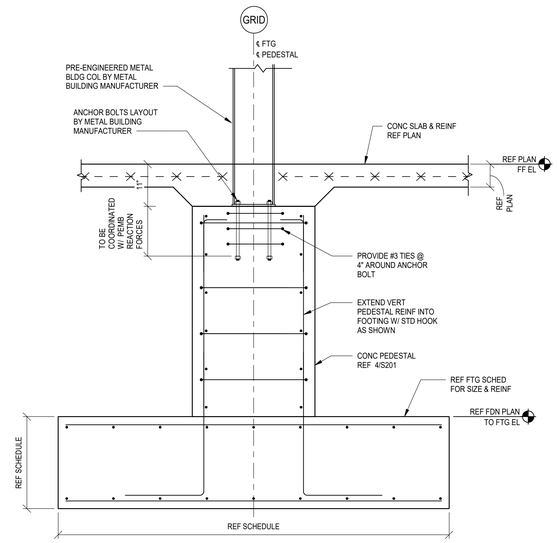
© 2022 OKW ARCHITECTS



1 EXTERIOR COLUMN DETAIL
3/4" = 1'-0"



2 SECTION AT EXTERIOR STEM WALL
3/4" = 1'-0"



3 INTERIOR COLUMN DETAIL
3/4" = 1'-0"

4 EXTERIOR COLUMN PEDESTAL DETAILS
3/4" = 1'-0"

5 DETAIL AT STAIRS
3/4" = 1'-0"

NOT FOR CONSTRUCTION

NO.	ISSUED FOR:	DATE
	PERMIT	06/27/2025

GEORGIA REGISTERED No. 29469 PROFESSIONAL ENGINEER GRANT COLLINS

06.27.2025

JFTE James F. Turner Engineers, L.P.
Consulting Engineers
8340 Meadow Road Suite 160
Dallas, TX 75231
TEL 214-750-2900
FAX 214-750-2902
FIRM # TBPE002989
Job Number: 55015.01

DRAWN HPL | DESIGN CW | QC GC | APPD GC

OKW Architects

OKW
Architects
600 West Jackson Blvd
Chicago, IL 60661
T 312.798.7700
F 312.798.7777
www.okwarchitects.com

CROWN MORELAND AVENUE MAINTENANCE SHOP

2750 MORELAND AVENUE
ATLANTA, GEORGIA

FOUNDATION DETAILS

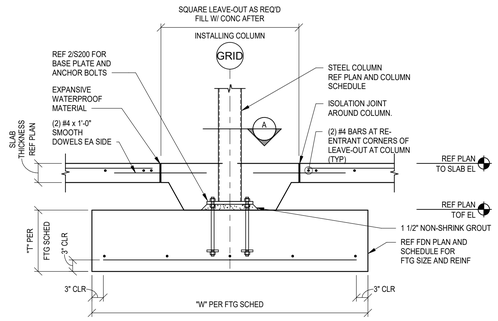
DRAWN BY: HL CHECKED: GC

DATE: 08/28/19

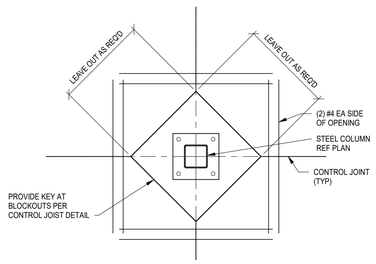
SHEET NO.: S201

PROJ. NO.: Project Number

© 2022 OKW ARCHITECTS



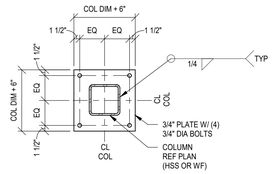
INTERIOR FOOTING AT COLUMN



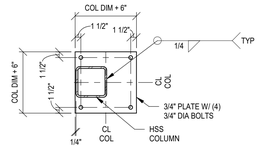
SECTION 'A'

1 TYPICAL COLUMN FOOTING

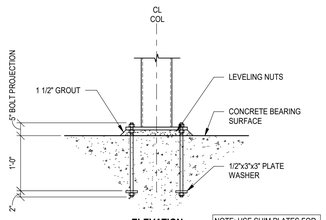
3/4" = 1'-0"



INTERIOR HSS BASE PLATE



PERIMETER HSS BASE PLATE

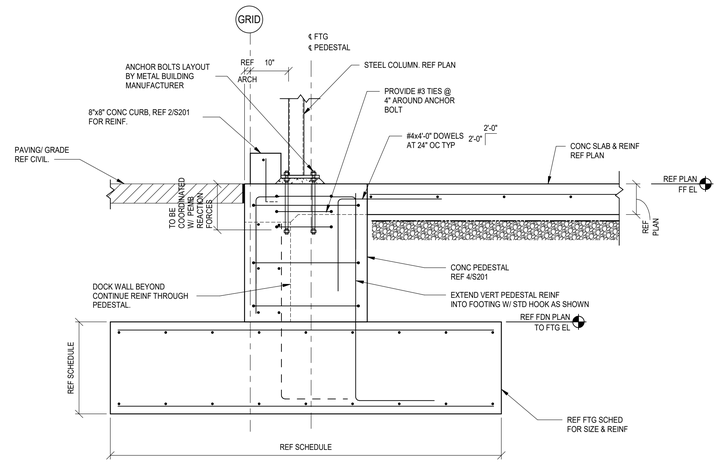


ELEVATION "TYPICAL"

NOTE: USE SHIM PLATES FOR LEVELING WHERE ONLY (2) ANCHOR BOLTS ARE SHOWN

2 BASE PLATE DETAIL

3/4" = 1'-0"



3 TYPICAL HSS COLUMN AT PERIMETER

3/4" = 1'-0"

NOT FOR CONSTRUCTION

NO.	ISSUED FOR:	DATE
	PERMIT	06/27/2025

OKW Architects

OKW
Architects

600 West Jackson Blvd
Chicago, IL 60661
T 312.798.7700
F 312.798.7777
www.okwarchitects.com

CROWN MORELAND AVENUE
MAINTENANCE SHOP

2750 MORELAND AVENUE
ATLANTA, GEORGIA

FOUNDATION DETAILS

GEORGIA REGISTERED No. 29468 PROFESSIONAL ENGINEER GRANT COURTNEY

06.27.2025

James F. Turner Engineers, L.P.
Consulting Engineers
5340 Alkahest Road Suite 160
Frisco, TX 75034
TEL 214-750-2900
FAX 214-750-2902
FIRM # REF001309

Job Number: 55015.01

DRAWN	HPL	DESIGN	CW	QC	GC	APPD	GC

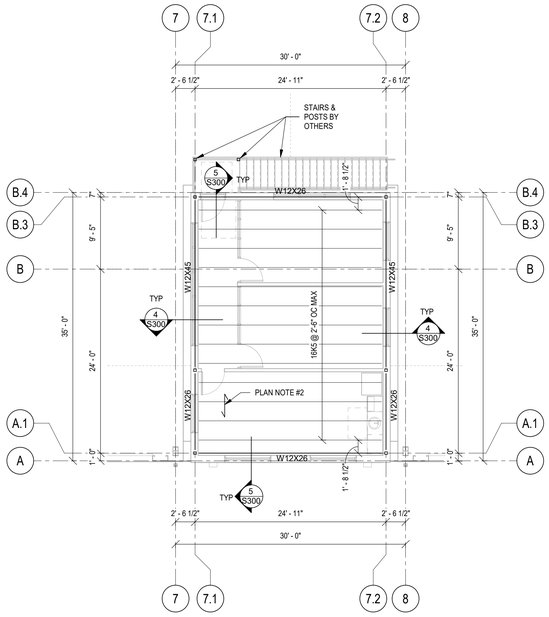
DRAWN BY: HL CHECKED: GC

DATE: 11/13/23

SHEET NO.: S202

PROJ. NO.: Project Number

© 2025 OKW ARCHITECTS



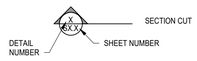
1 MEZZANINE FLOOR PLAN

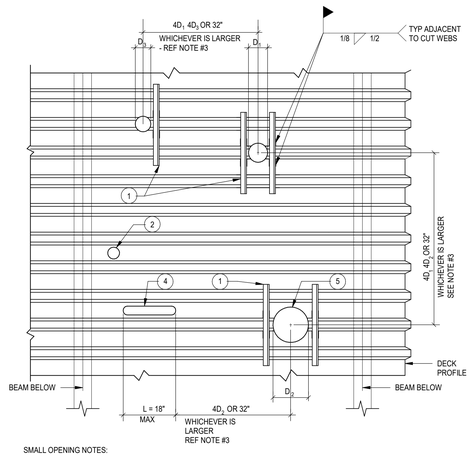
1/8" = 1'-0"

FRAMING PLAN NOTES:

1. ALL DIMENSIONS AND ELEVATIONS SHALL BE VERIFIED WITH THE ARCHITECTURAL DOCUMENTS.
2. FINISHED FLOOR ELEVATION = 11'-0" AND SLAB SHALL BE 2" LIGHTWEIGHT CONCRETE OVER 1.0 C x 22 GA METAL DECK W/ 5/8" Ø WELDS IN 36/7 PATTERN (3" TOTAL THICKNESS - 3 SPAN MINIMUM). REINFORCED w/ 6x6- W2.9xW2.9 WELDED WIRE FABRIC. DO NOT SAWCUT JOINTS INTO ELEVATED CONCRETE SLAB ON DECK.
3. ALL BEAM CONNECTIONS TO BE DESIGNED BY FABRICATOR WITH BOLTED AND/OR WELDED AISC CONNECTIONS FOR 50% MAX BEAM SHEAR CAPACITY. NO MOMENT CONNECTIONS.
4. JOIST MANUFACTURER TO PROVIDE ADEQUATE CLEARANCE FOR 8" DUCT THROUGH JOIST WEBS. REF MECH FOR LOCATIONS.

LEGEND:



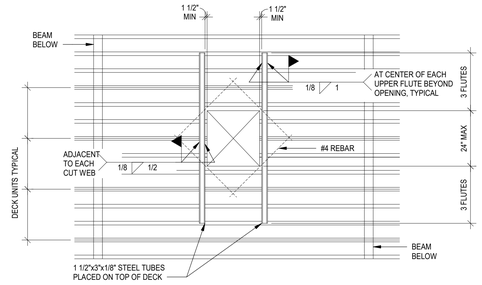


SMALL OPENING NOTES:

1. REINFORCED OPENINGS WITH L 1 1/2" X 1 1/2" X 1/4" AS SHOWN. ANGLES SHALL BE PLACED ON TOP OF DECK AND MUST EXTEND ACROSS (3) WEBS PAST OPENINGS.
2. HOLES LESS THAN 6" IN DIAMETER AND CUTTING NO MORE THAN (1) WEB REQUIRE NO REINFORCEMENT.
3. IF DIMENSIONS BETWEEN OPENINGS ARE LESS THAN THE MINIMUM DIMENSION NOTED, THE OPENING GROUP SHALL BE CONSIDERED AS A SINGLE HOLE AND REINFORCED AS FOR A LARGER OPENING.
4. PRIOR TO CONCRETE POUR, SMALL OPENINGS SHOULD BE BLOCKED OUT AND DECKING LEFT INTACT. AFTER CONCRETE HAS CURED, BLOCKOUT CAN BE REMOVED AND DECKING IN THE AREA OF THE HOLE REMOVED.
5. MAXIMUM HOLE MAY BE CUT NO MORE THAN:
   18" FOR TYPE 'B FORMLOK'
   24" FOR TYPE 'W2 OR W3 FORMLOK'
   FOR LARGER OPENINGS, CONTACT ENGINEER FOR HEADER BEAM REQUIREMENTS

2 FLOOR DECK REINF AT SMALL OPENINGS

3/4" = 1'-0"

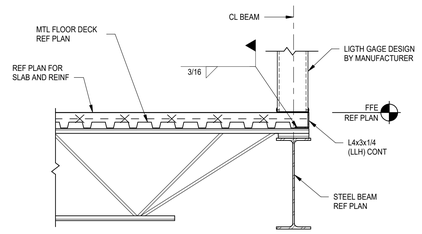


LARGE OPENING NOTES:

1. IF THE OPENING OR GROUP OF OPENINGS OCCURS IN ONE DECKING UNIT, THE OPENING OR OPENING GROUP MAY BE CUT PRIOR TO POURING OF CONCRETE.
2. IF, AS SHOWN IN THE DETAIL ABOVE, THE OPENING OR OPENING GROUP CUTS THROUGH TWO DECKING UNITS, THE DECKING SHALL NOT BE CUT UNTIL CONCRETE HAS BEEN PLACED AND CURED.
3. AT THE TIME OF POURING, SUITABLE SLEEVES OR BULKHEADS SHALL BE PLACED AROUND THE OPENING.
4. ADD REBAR AT CORNERS OF OPENING.
5. WHEN THE MAXIMUM DIMENSION OF AN OPENING OR OPENING GROUP EXCEEDS 24", CONTACT ENGINEER FOR HEADER BEAM REQUIREMENTS.

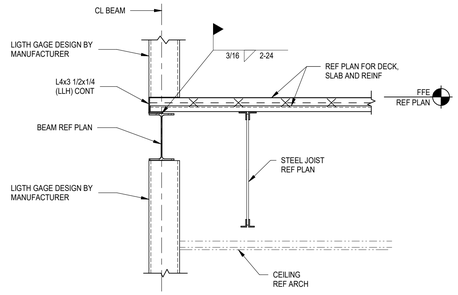
3 FLOOR DECK REINF AT SMALL OPENINGS

3/4" = 1'-0"



4 SECTION AT SLAB EDGE

3/4" = 1'-0"



5 SECTION

3/4" = 1'-0"

CROWN ENTERPRISES LLC

NOT FOR CONSTRUCTION

NO.	ISSUED FOR:	DATE
	PERMIT	06/27/2025

OKW Architects

OKW Architects
600 West Jackson Blvd
Chicago, IL 60661
T 312.798.7700
F 312.798.7777
www.okwarchitects.com

CROWN MORELAND AVENUE MAINTENANCE SHOP

2750 MORELAND AVENUE
ATLANTA, GEORGIA

MEZZANINE FRAMING PLAN & DETAILS

GEORGIA REGISTERED No. 29469 PROFESSIONAL ENGINEER GRANT COUNTY

06.27.2025

James F. Turner Engineers, L.P.
Consulting Engineers
4340 Meadow Field Suite 160
Dallas, TX 75223
TEL 214-750-2900
FAX 214-750-2902
FIRM # F8703/7329

Job Number: 55015.01

DRAWN	HPL	DESIGN	CR	QC	GC	APPD	GC

DRAWN BY: HL CHECKED: GC

DATE: 11/13/23

SHEET NO.: S300

PROJ. NO.: Project Number

© 2022 OKW ARCHITECTS